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**UTILITIES PLANNING SERVICES**

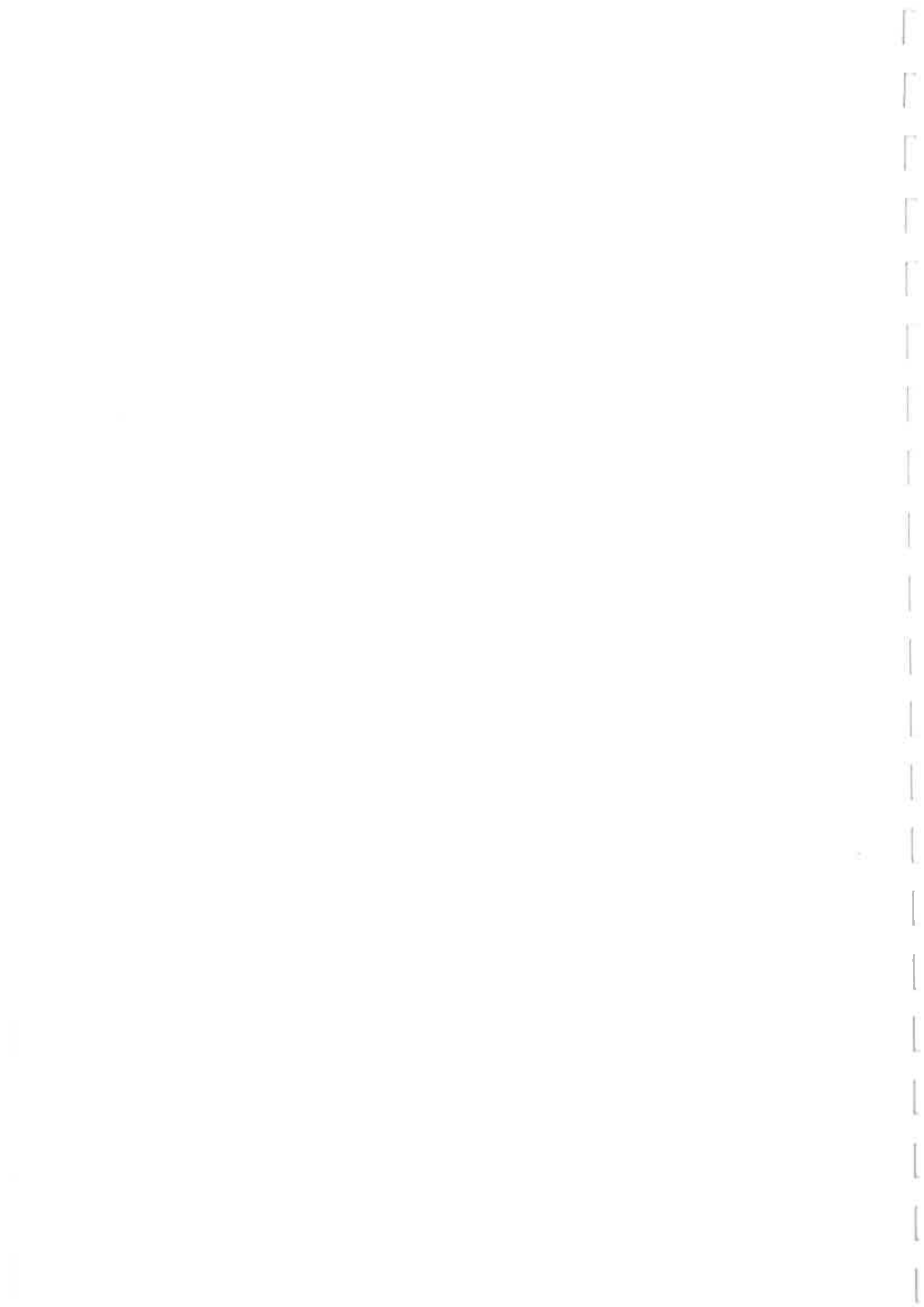
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**JOHNSTONS CREEK SWC 55  
CAPACITY ASSESSMENT**

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December 1995

Sydney  
**WATER**



# JOHNSTONS CREEK SWC 55 CAPACITY ASSESSMENT

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# SECTION 1

## OVERVIEW

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### PURPOSE

This report assesses the current quantitative performance of Sydney Water's Johnstons Creek SWC 55 and gives an estimate of the impact of simulated urban consolidation on that performance.

### SYSTEM DESCRIPTION

#### Location

The Johnstons Creek Stormwater Drainage System and Area is located to the west of the City of Sydney. Its catchment is one of five declared Sydney Water Drainage Areas within the Johnstons Bay Stormwater Group (SWG) 7 as shown in **Figures 1-1 and 1-2**.

The system serves a resident population of about ..... people in the Sydney suburbs of Annadale, Camperdown, Enmore, Glebe, Newtown and Stanmore as shown in **Figure 1-3**. The Johnstons Creek Drainage Area falls in three local government areas: Leichhardt, Marrickville and South Sydney; a breakdown is shown in **Table 1-1** below. Johnstons Creek flows into Rozelle Bay, an arm off Johnstons Bay. It is tidal for about 0.5 km upstream of its outlet.

**TABLE 1-1 LOCAL GOVERNMENT AREAS IN JOHNSTONS CREEK DRAINAGE AREA 55**

<b>Municipality</b>	<b>Area (ha)</b>	<b>Proportion %</b>
Leichhardt	168.0	36.5
Marrickville	183.5	39.9
South Sydney	108.5	23.6
<b>TOTAL</b>	<b>460.0</b>	<b>100.0</b>

### Land Zoning and Use

The drainage area is fully urbanised. Local government land zoning detail captured by the Utilities Planning Services (UPS) geographic information system (GIS) reveals the breakdown shown in **Table 1-2**.

TABLE 1-2 LAND ZONING IN JOHNSTONS CREEK DRAINAGE AREA 55

Zoning	Area (ha)	Proportion %
Residential	221.9	48.2
Special Use	135.9	29.5
Industrial	44.0	9.6
Open Space	39.1	8.5
Commercial/Business	19.0	4.1
<b>TOTALS</b>	<b>460.0</b>	<b>100.0</b>

The housing density is generally medium with some pockets of multi-storey units. The peak equivalent dwelling density in any Australian Bureau of Statistics (ABS) *collector district* in the drainage area is about 54 equivalent dwelling units per hectare (edu/ha). The number of equivalent dwelling units represents the number of ground floor units in high rise areas, that is, their *footprint*, and has been estimated from information provided in the 1991 Census. The lowest dwelling density is about 22 edu/ha, typically ranges between 24 and 40, whilst the average for the drainage area is about 32 edu/ha.

The special use areas include Sydney University and three major Sydney hospitals. Sydney Uni has at least five ovals and other areas of open space. Some of these fields may have the potential to retard flows to the system and perhaps already do so with the more significant storm events. In addition to a railway corridor, some 29 ha is reserved for a major urban motorway. It is assumed that not all the reserved area will be road surface; a proportion impervious of 75 percent has been adopted. Probable RTA on-site controls have been ignored.

The main open space areas have little impervious areas excepting Harold Park Paceway which is assumed to be 30 percent impervious. The other areas of note include Bicentennial Park along the foreshores of Rozelle Bay, Jubilee Park, Federal Park, Camperdown Park, Camperdown Memorial Rest Park and O'Dea Reserve. The AR&R coefficients of runoff for such areas are considered to be very conservative and are believed to over-estimate the peak. However, their adoption may overcome the need for partial area considerations.

The industrial area is centred on Parramatta Road at Camperdown and a small pocket on the southern side of the railway near Newtown. The average percent impervious is assumed to be 85 percent. Commercial/business areas at Annandale, Camperdown, Newtown and Stanmore are assumed to be 90 percent impervious.

The average equivalent percent impervious at the system's outlet for the existing drainage area, with the freeway in place, weighs in at 63 percent.

### Dimensional Detail/ Summary

Sydney Water owns and operates nearly 10.7 km of the drainage system in this catchment. This includes about 1.0 km of main channel relief now generally known as the Cardigan Street Branch which operates independently off the main channel. As a result of amplifications in 1928 and 1965, the overall length of stormwater conduit owned by Sydney Water is nearly 11.8 km. A breakdown by type and size is shown in Table 1-3.

TABLE 1-3 SUMMARY OF SYDNEY WATER'S ASSETS IN JOHNSTONS CREEK DRAINAGE SYSTEM

Type of Conduit	Typical Size Range mm	Length km	Percent
Open Channel	15545 x 1625 to 5270 x 2970	1.64	14
Covered Channel	2820 x 1370 to 1040 x 865	4.47	38
Boxed	1830 x 1370 to 915 x 915	0.67	6
Pipe	1500 to 750	3.84	33
Pipe	675 to 300	1.18	10
<b>Total</b>		<b>11.79</b>	

### Operational Aspects

The assessment of the performance of the system includes a number of operational assumptions.

- The Gladstone St Branch is blocked off at node VB1 to divert all upstream flows to the Cardigan St Branch.
- The two parallel conduits of the amplified section of the Stanmore Branch

handle the total flow from the sub-catchment in the ratio of their capacities irrespective of physical connections.

- The balance lines allow the two conduits of the Fowlers Creek Branch to handle the total flow from the sub-catchment in the ratio of their capacities.
- The flow from the Kingston Rd Branch sub-catchment is similarly proportioned on the basis of its capacity to that of a [presumed] council owned relief pipe to another point on the Cardigan St Branch.

## METHODOLOGY

### Aim of the Assessment

The aim of the analysis is to assess the performance of the various elements of the drainage system in terms of the average recurring design storm required for the corresponding peak flow to exceed the hydraulic capacity of the drainage path, ie channel plus any overbank capacity. Upstream attenuation is ignored unless integral to operation of the system. This capability is referred to as the Storm Event Capacity (SEC) afforded by the reach and represents the threshold of possible undesirable overland flows and/or flooding beyond the recognised drainage path.

The SEC is compared to design standards typically adopted in the past for the use of the land through which the channel passes. This benchmark may be different from the original design and should not be confused with the higher demand for flood damage prevention with 100 year ARI (average recurring interval) storms.

### Methodology

A separate report *Stormwater Capacity Assessment - Methodology* will provide full documentation of the approaches, methodology and assumptions used for the analyses. The following points provide an outline to the report.

- A spreadsheet approach founded on updated Regional databases and applicable data from previous evaluations.
- Flow rates are estimated by the Rational Method as outlined in Australian Rainfall and Runoff (AR&R), 1987, in particular:
  - Intensity-Frequency-Duration Design Rainfall, with the aid of Glenn McDermott's *Rainman*, to derive easy to use relationships appropriate for each drainage area; and



- Coefficients of Runoff based on percent impervious.
- The percent impervious in residential areas is based on the equivalent dwelling density estimated from data in the 1991 Census, for example, 40 percent impervious (including share of public roads) for 10 edu/ha, 72 percent impervious for 30 edu/ha and up to 90 percent impervious for densities of and exceeding 50 edu/ha. Where the 1991 density is less than 9, this minimum value is adopted.
- Hydraulic capacity using the Manning Formula and AR&R methods for composite roughness and compound sections. Manning roughness parameters generally ranges from 0.013 for concrete pipe and brick oviform to 0.014 for other concrete sections and brick walls.
- Circular sections and pipes are assumed to be flowing full. Culverts and the shorter reaches of covered channel are treated as open to establish the upper bound capacity prior to the impact of soffit friction losses. For the longer covered and boxed reaches, hydraulic capacities are based on flowlines below the soffit (generally 98 percent of the height unless the cover is clear above the original coping) whereas previous evaluations may be based on closed sections. Overbank capacity within the drainage easement is included where available and is given as a multiple of the formal constructed drain.
- The SEC is determined by looking up the storm event that causes a peak flow equal to the hydraulic capacity. Such events are provided as a range, viz: below 6 months, 6 months to a year, 1-2, 2-3 and so on to 9-10, 10-12, 12-15, 15-20, 20-25, 25-50, 50-100, and greater than 100 year. The results are grouped into one of six SEC Rating Groups which are featured on GIS based figures. Details are summarised in a series of charts and tables with supporting calculations.
- The spreadsheet can easily include the impacts of increased dwelling density.

## SECTION 2

# PERFORMANCE RESULTS

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### Desirable Design Standard

To assess the performance only, each reach of the system has been assigned one of three design ARI according to the zoning/use of the surrounding land, typically:

- 5 year ARI for low density residential and open space areas;
- 10 year ARI for business, commercial and industrial areas, intensely developed residential areas, and local access road culverts; and
- 20 year ARI for intensely developed business, commercial and industrial areas, major road and railway culverts, and the wider service corridors where the channel is obviously a trunk drain as defined by AR&R, though the 100 year ARI may be more appropriate for highways, railways and floodways.

## RESULTS

### Summary for System

**Table 2-1** lists dimensional detail, hydraulic capacity, peak flow corresponding to 5 year ARI rainfall intensity-duration, the consequent storm event capacity (SEC) and the assigned design ARI for each reach. For the Johnstons Creek drainage system, the SEC ranges from frequent events (twice per year) to rare events (once in 100 years).

**Figure 2-1** features the SEC Group Rating for each reach against a backdrop of the land zoning and use. The figure highlights the poor rating for the open main channel reaches downstream of the Orphan School Creek Branch, the many reaches of the this Branch and its Hockey Field Sub-branch, and for the Stanmore Branch in particular.

**Table 2-2** provides a dissection of the proportion having a certain event capacity, ie, the proportion satisfying certain events. This is given for the system as a whole, and by each one of the three design ARI.

This performance is shown graphically in **Figure 2-2**. The line graph shows the proportion able to satisfy or transport storm events up to 20 year ARI. In particular, the graph demonstrates that only about half of the system copes with the 5 year ARI storm, about a quarter cannot handle a two year storm though a third of the system copes with a 10 year ARI event and nearly 20 percent copes with a 20 year ARI storm.

However, the bar chart of **Figure 2-2(b)** reveals that much of these meagre capabilities are not in areas where they are required. For example, of the 18 percent good for a 20 year storm, none occurs where this is desirable. Similarly, of the 31 percent that can handle a 10 year design storm, only about two thirds or 20 percent of the system is located where desirable. And yet again, in only 22 percent of the system out of 54 percent of the system where a 5 year ARI is desirable, is this minimal demand satisfied.

**Tables 2-3A, 2-3B, 2-4 and 2-5** provide the assumptions and supporting calculations to estimate the hydraulic capacity, overbank capacity, 5-year peak flow and the look-up table for this drainage system. Overbank capacity is based on visual observations of site dimensions and shape. [Table sheet numbers correspond with **Table 2-1**.]

### Component Summaries

In the following paragraphs, reference to numbers of years refers to the ARI in years of a design rainfall event.

**Main Channel - Open Portion.** There is little consistent overbank capacity that can be relied on currently. However some potential exists to improve this relatively inexpensively for the first 1.1 km stretch of the system up to the Orphan School Creek. In this stretch, the SEC is relatively poor, ranging from the 6 month to the 3 year ARI. At the upstream end of this section there is a pocket of town houses with floor levels little higher than the coping of the adjacent channel.

Probable/ possible attenuation/ retardation within the grounds of Sydney University may improve these performances marginally.

Upstream of Orphan School Creek, the SEC improves, though only half of this 0.5 km section approaches a rating of 10 year ARI.

**Main Channel - Covered Portion.** Downstream of the Cardigan St Branch (Main Channel Relief), the covered main channel is rated at 5 years. Upstream of this Branch, the channel performs generally as desired with the exception of an 140 m reach.

**Federal Park Branch.** The 80 m amplified section performs as desired at around the 10 year ARI. However, the upper 100 m reach is rated at only 1-2 years, though improvements may be part of major future RTA roadwork.

**Lillie Street Branch.** The performance of this 1 km branch is generally very poor to poor, with event capacity ranging from less than 1 year ARI to 3-4 years.

**Piper Street Branch.** This short 70 m branch performs very poorly and has a SEC of 6 months to 1 year. However, it is located in the corridor reserved for major roadwork which, if it proceeds, is likely to result in this 600 mm pipe being replaced by something a little larger.

**Taylor Street Branch.** This is another short branch (100 m) in the roadway corridor which currently performs very poorly at 1-2 year ARI.

**Orphan School Creek Branch.** This branch (1.6 km) and its sub-branches (totalling a further 1.7 km) represent a significant proportion of the system. The main branch is generally rated very poorly from its outlet into Johnstons Creek up to the Medical School within Sydney Uni. This event capacity for this section is 1 to 3 years ARI. Elsewhere, the event capacity generally exceeds 25 years.

Its sub-branches, all within the grounds of Sydney Uni, perform variously. The **Hockey Field Sub-branch** is rated very poorly at 1-2 years, though this must encourage some retardation from the No 2 Oval. The generally parallel **Physics School Relief Sub-branch** copes well. A further **sub-branch off the Physics School Relief**, rated at under 1 year, presumably behaves as an outlet control to retard flows from St Paul's Oval.

The **St Andrews College Sub-branch** is rated progressively from 1-2 to 20-25 years ARI over its short length of 160 m from Orphan School Creek.

**Gehrigs Creek Branch.** This short 140 m branch easily copes with an 100 year storm from its industrial catchment.

**Cardigan Street Branch.** This main channel relief system which also intercepts the Fowlers Creek and Kingston Rd Branches and most of the Gladstone St Branch, generally performs well with SEC ranging from 10 to 50 year ARI. However, the SRA owned portion under the rail, linking the Gladstone St Branch, performs a little below desired levels at 3-4 years.

The **Gladstone Street Branch**, is also rated well, except for an 8 m section, rated at 2-3 years, that links most of the sub-catchment to the Cardigan St Branch.

**Fowlers Creek Branch.** This amplified sub-branch off the Cardigan St Branch has event capacity ranging from 10 to 15 years ARI, appropriate for the industrial zoning through which the conduit passes.

**Kingston Road Branch.** This short 40 m sub-branch intercepted by the Cardigan St Branch, with an ARI event capacity of 3-4 years in conjunction with a council owned relief drain, performs a little poorer than desirable. However, it is also located in the RTA corridor reserved for roadway.

**Stanmore Branch.** Despite a 1928 amplification for most of its length, this 1.0 km branch performs poorly with SEC ranging from 2 to 4 years.

### Impact of Urban Consolidation

In comparison to many other Sydney suburbs, the average dwelling density, at 32 equivalent dwelling units per hectare, is already significant. It is unlikely that the character of the residential areas will be allowed to change significantly in the future.

Whilst, in theory, it is possible to stack more people into the area, it is unlikely to lead to measurable practicable difference in storm runoff. Consequently, simulated scenarios have not been examined in detail. A minimum 40 edu/ha across all residential areas has the following impact on the overall performance in terms of the proportion of the system handling a:

- 2 year ARI design storm - no change on existing situation;
- 5 " " " " - down 1 percent; and
- 10 " " " " - no change.

# **APPENDIX**

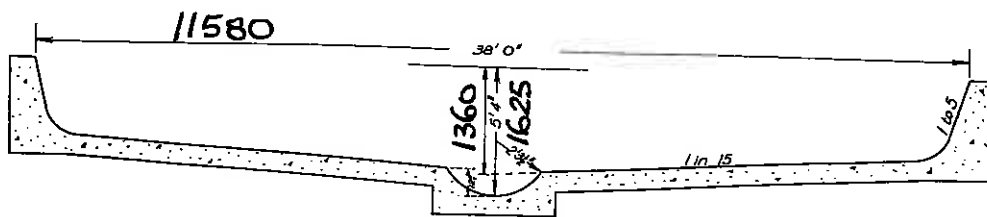
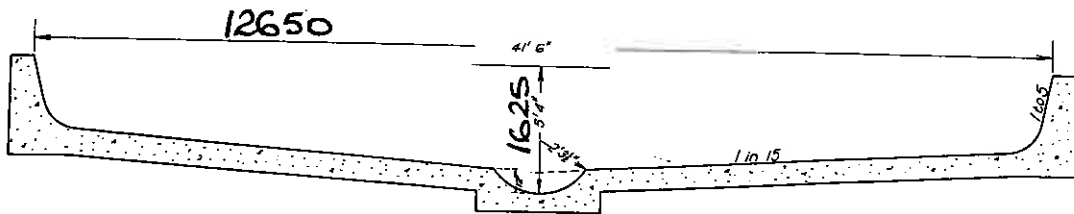
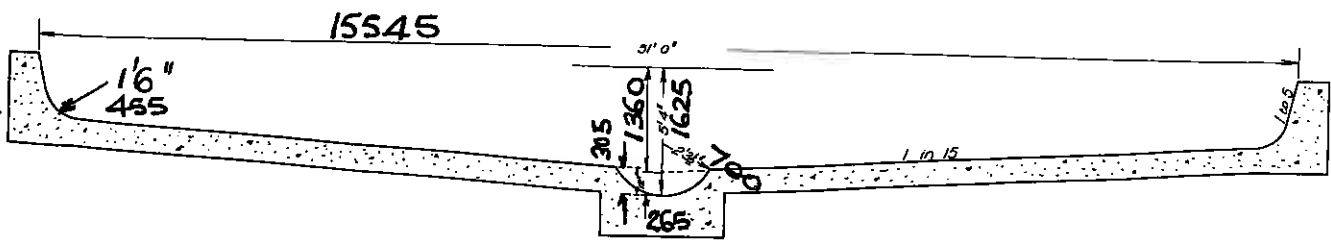
## **CHANNEL CROSS-SECTIONS**

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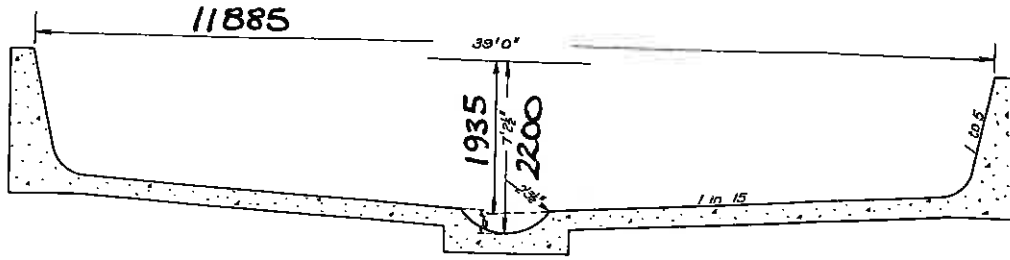
# APPENDIX

## CHANNEL CROSS-SECTIONS

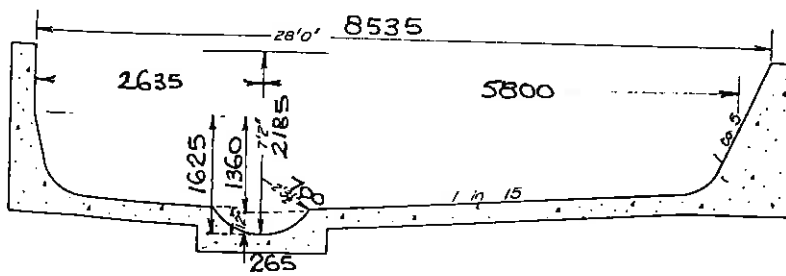
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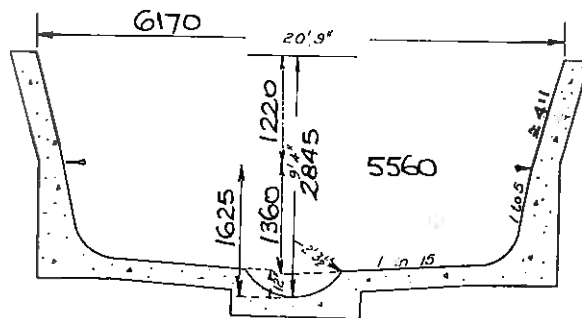
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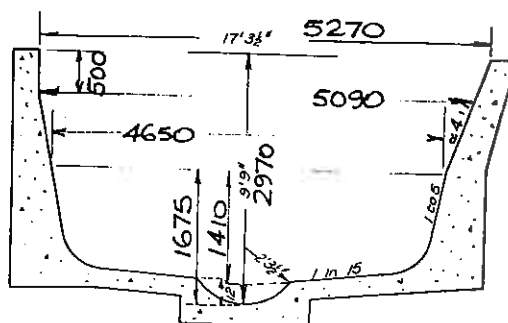
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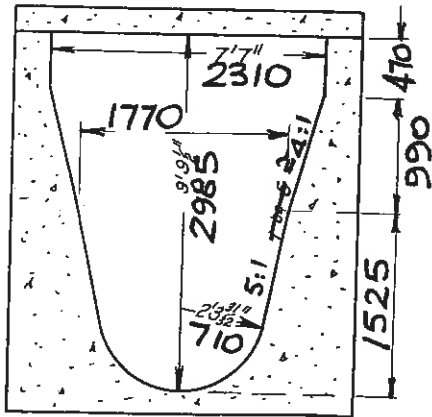
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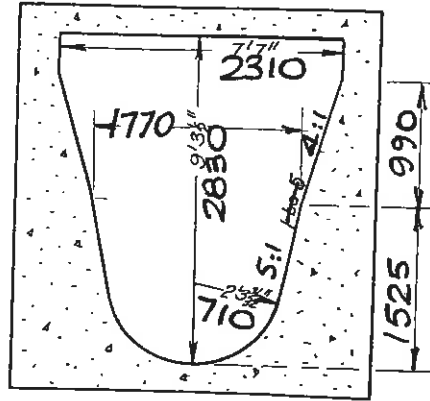
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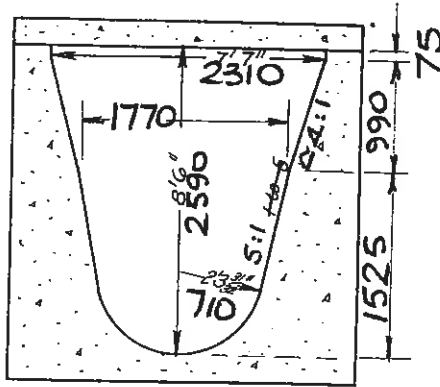
JOHNSTONS CREEK MAIN CHANNEL



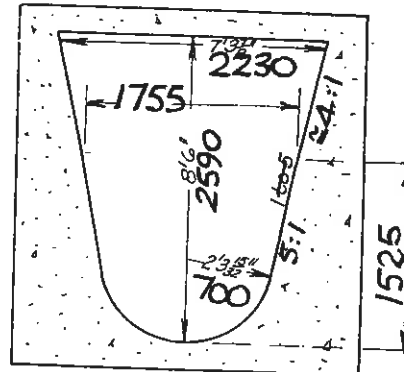
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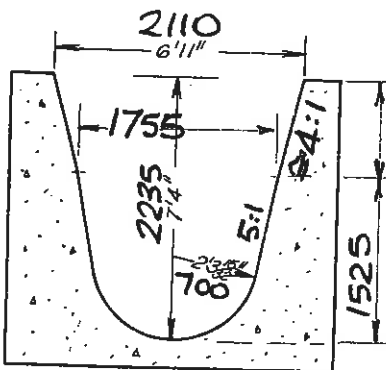
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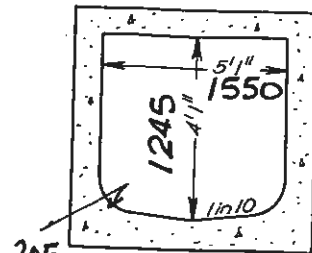
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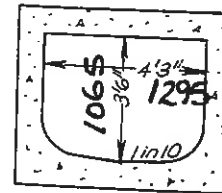
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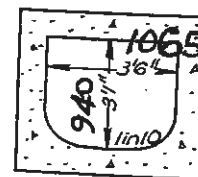
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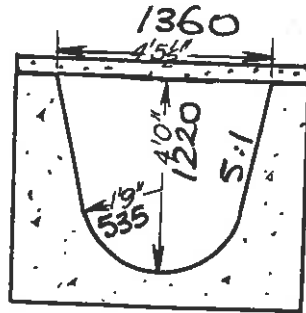


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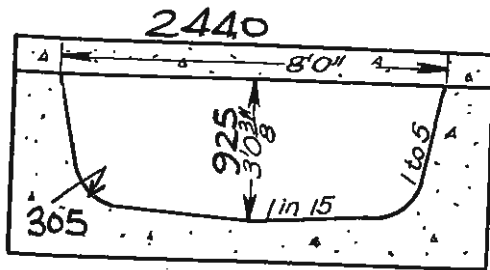
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FEDERAL PARK BRANCH 55A



BI-B2

LILLIE BRIDGE BRANCH 55B



D3-D4

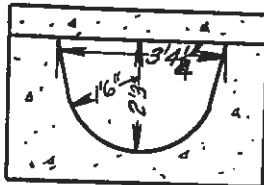
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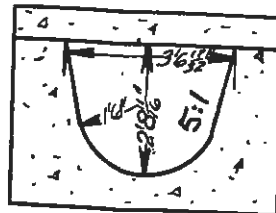
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TAYLOR STREET BRANCH 55D

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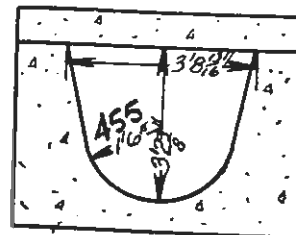


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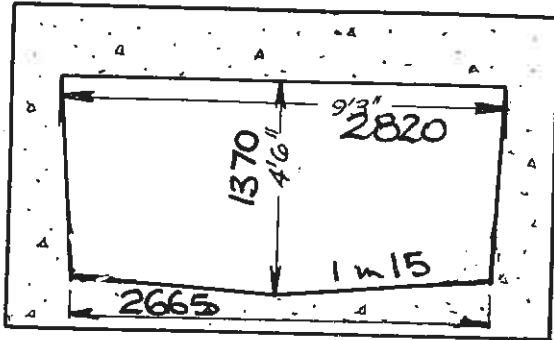


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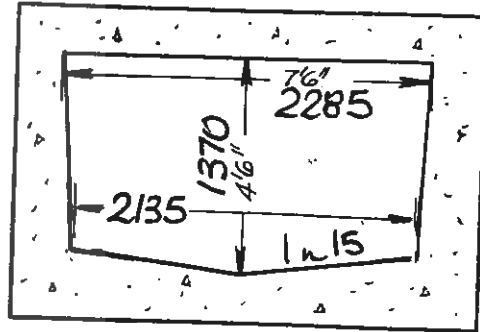
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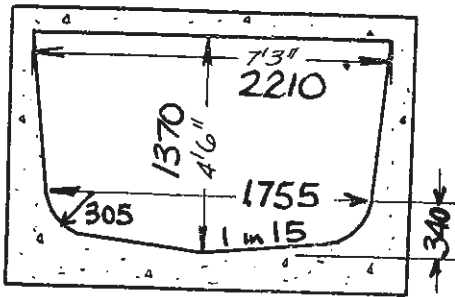
ORPHAN SCHOOL CREEK BRANCH 55E



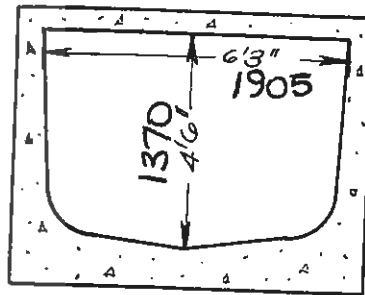
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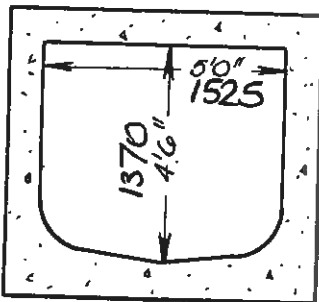
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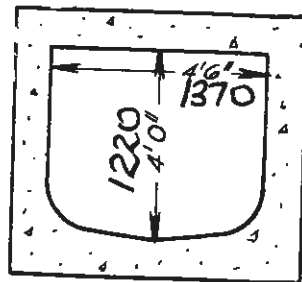
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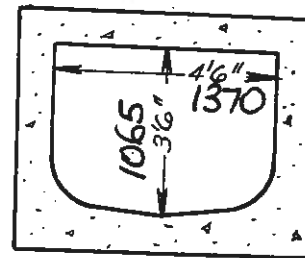
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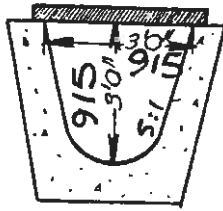


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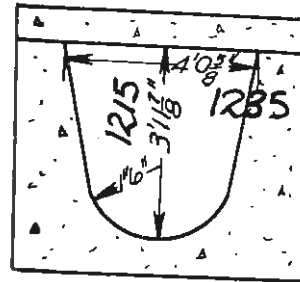
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ORPHAN SCHOOL CREEK BRANCH - HOCKEY FIELD SUB-BRANCH 55G



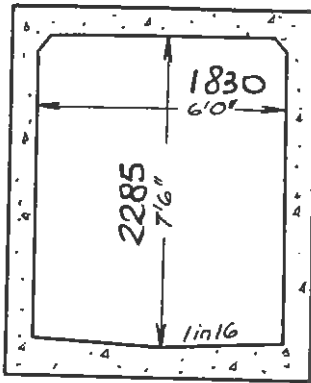
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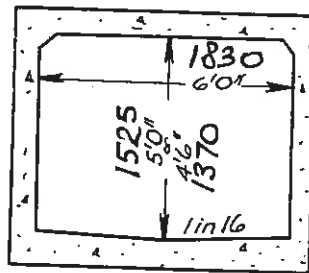


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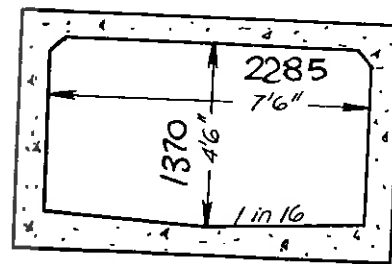
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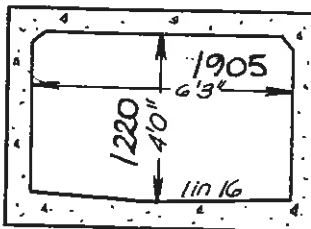
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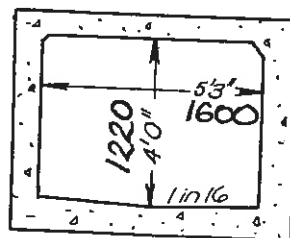
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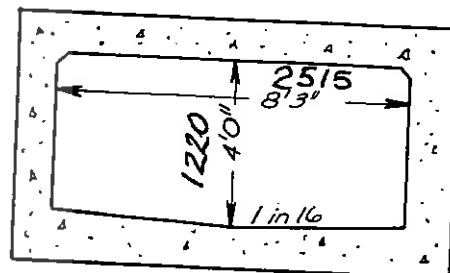
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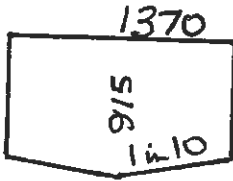


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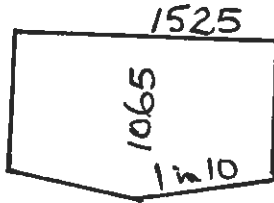


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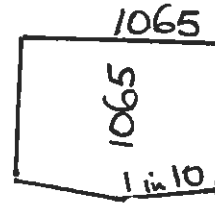
GLADSTONE STREET BRANCH 55U



Z-VB1

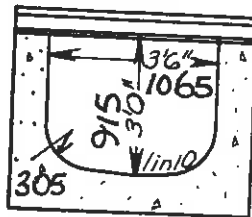


VB1-VC



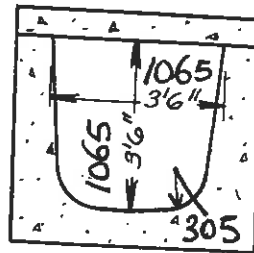
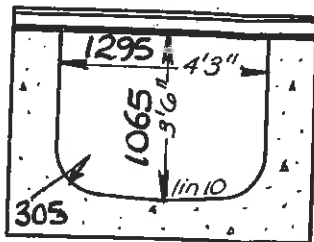
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FOWLERS CREEK BRANCH 55R

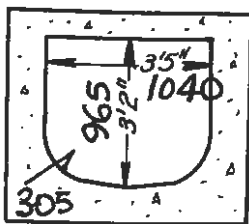


Q66-Q67

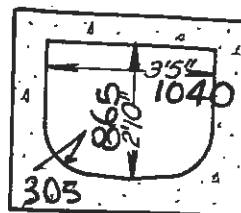
STANMORE BRANCH 55S



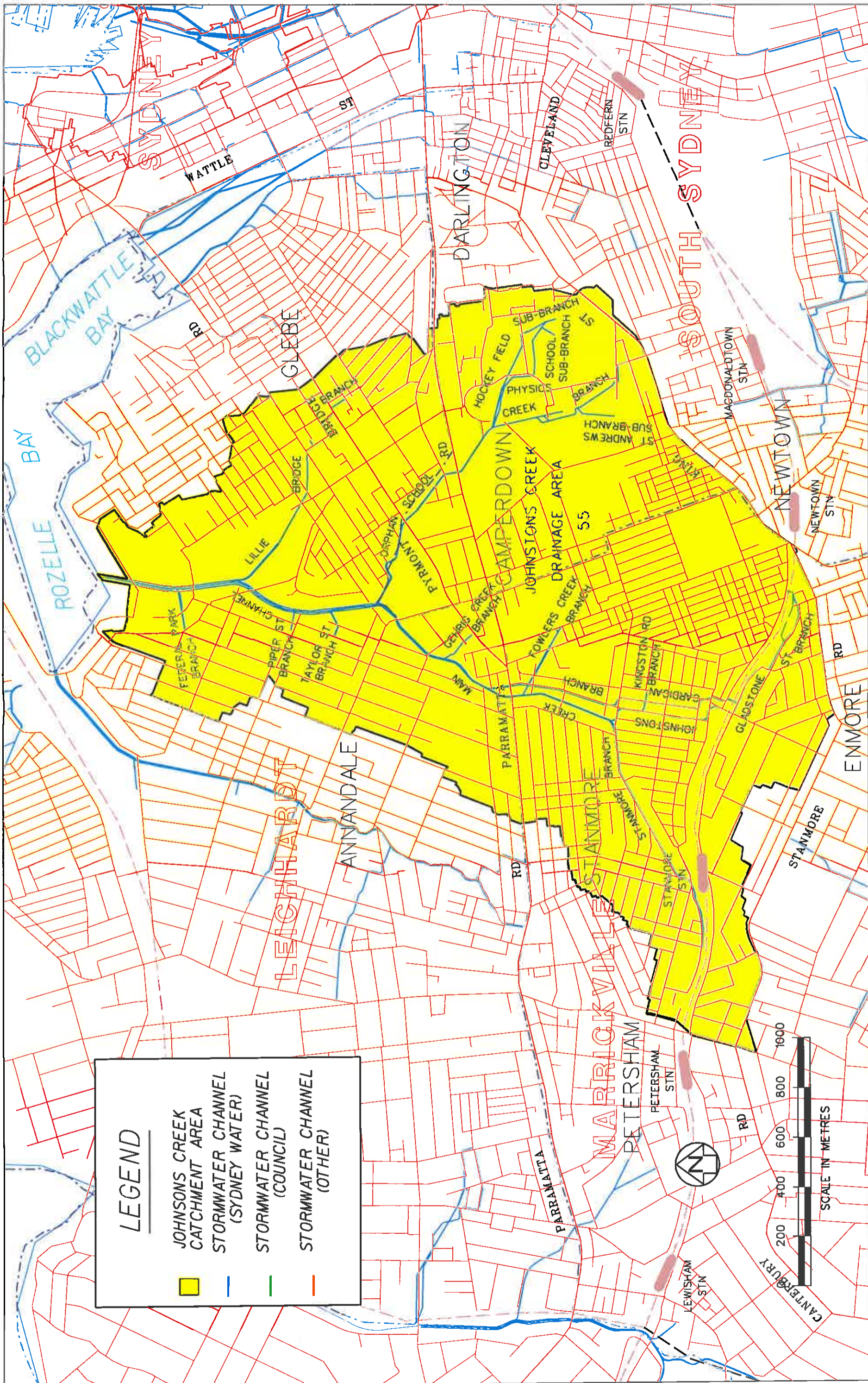
R-R71



R72-R73



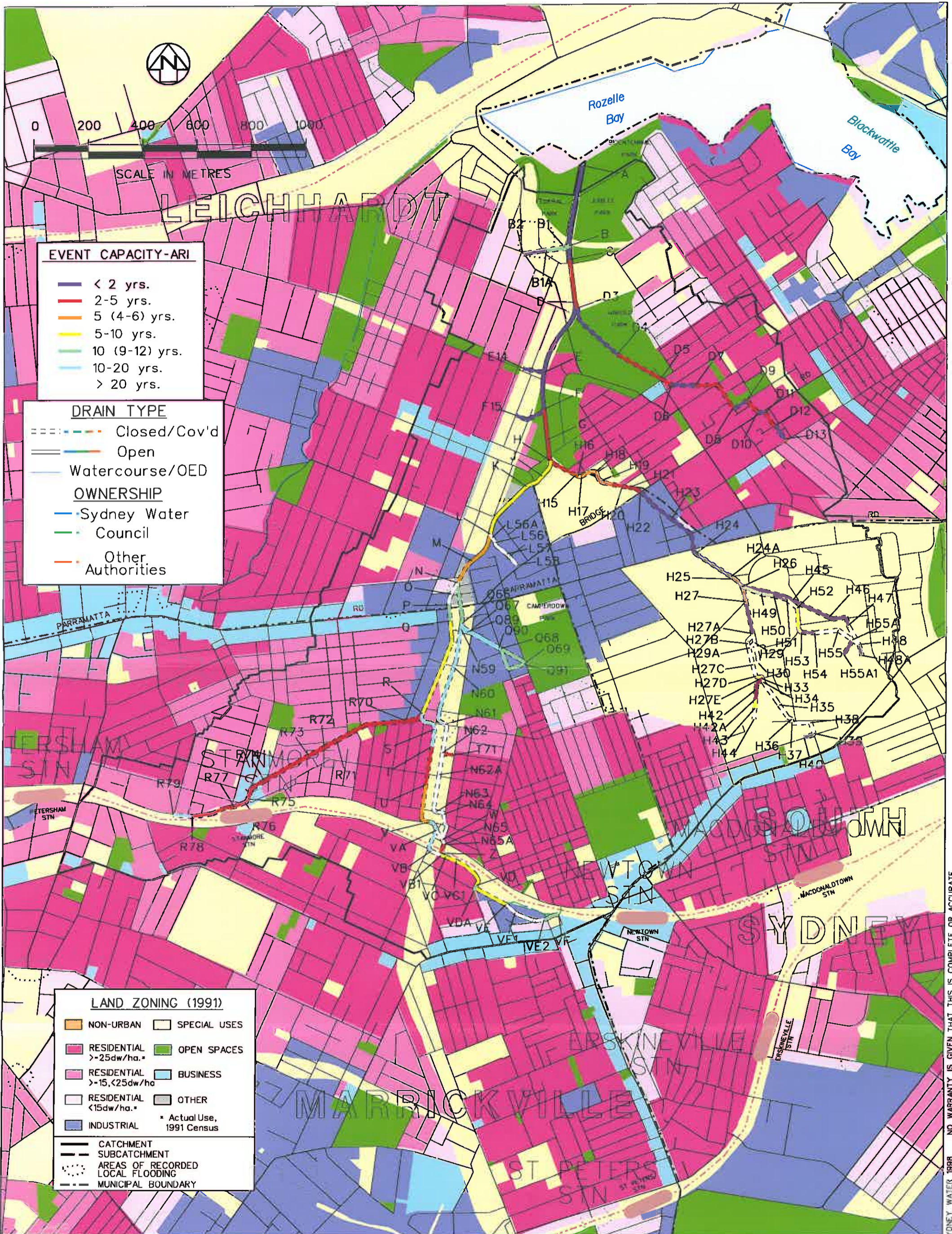
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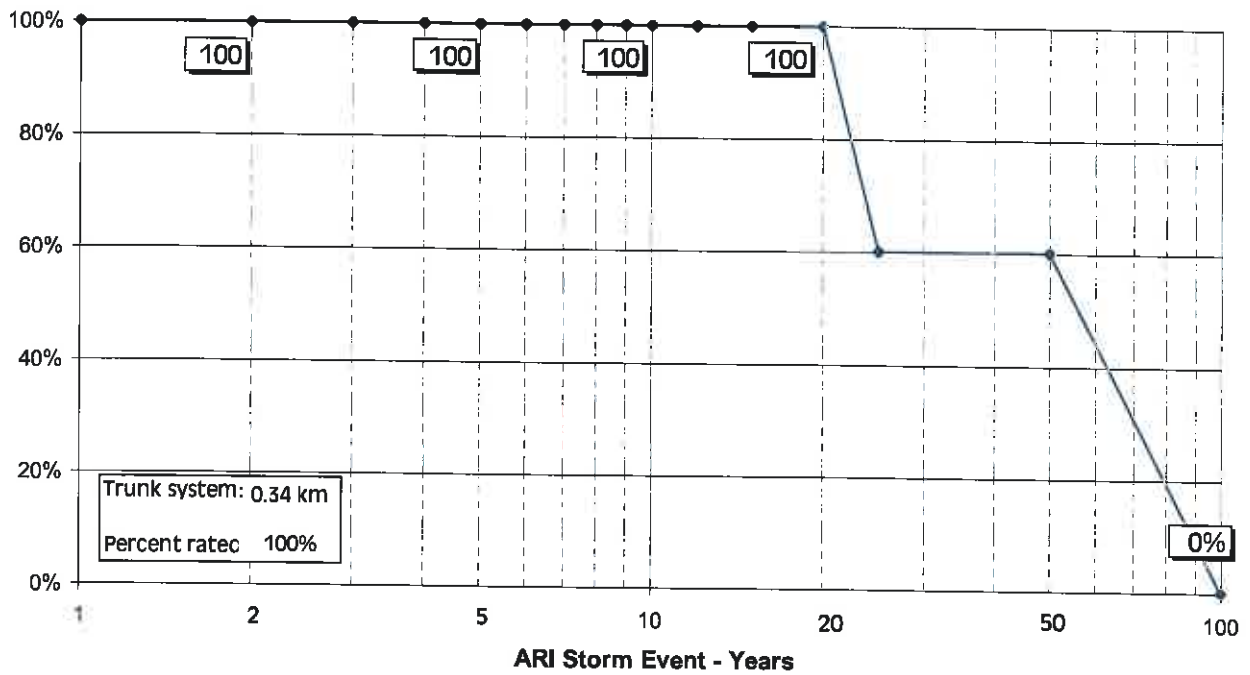
**JOHNSTONS CREEK SWC 55  
 CAPACITY ASSESSMENT**

**FIGURE I-3  
 DRAINAGE AREA  
 AND BRANCHES**

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(a) PERCENT OF RATED DRAINAGE SYSTEM ABLE TO HANDLE ARI STORM EVENT



(b) DITTO, GROUPED BY CURRENT LAND ZONING/ USE & STORM EVENT CAPACITY

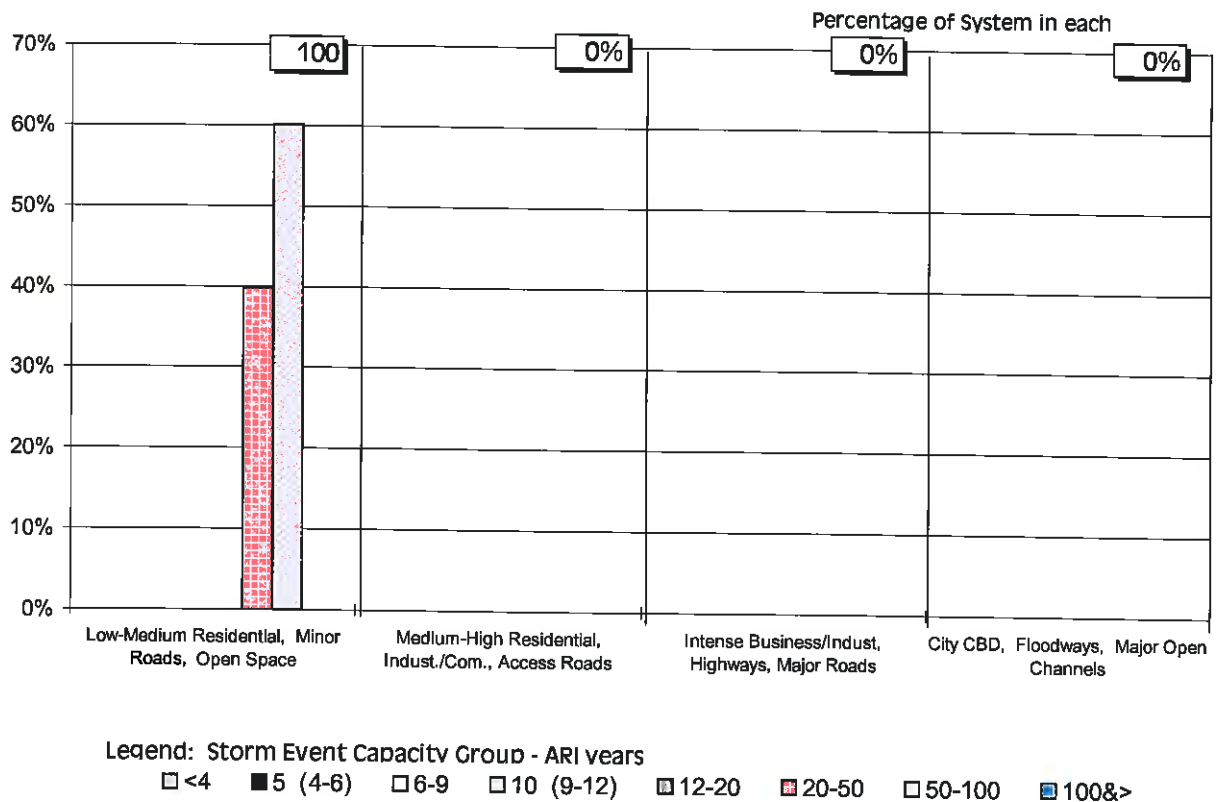


FIGURE 2-2. PERFORMANCE SUMMARY FOR KING GEORGE PARK SWC 24



**TABLE 2-2. STORM EVENT CAPACITY DISSECTION - stormwater Capacity Assessment  
JOHNSTONS CREEK SWC 55**

Type 20 type 20	Totals						Landuse Design ARI 5 year			Landuse Dsg ARI 10y			Landuse Dsgn ARI 20y							
	DesARI	SEC	Equivalent Leng	Cum %	Satisfy	Cum %	DesARI	Section	Cum	Equivalent Length (m)	Cum	Satisfy	Cum %	DesARI	Section	Cum	Equivalent Length (m)	Cum	Satisfy	Cum %
0.5	Below 5	63,400	63,400	100%	100%	100%	5	21,133	21,133	100%	100%	100%	20	21,133	21,133	100%	20	21,133	21,133	100%
1	0.5-1	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
2	1-2	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
3	2-3	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
4	3-4	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
5	4-5	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
6	5-6	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
7	6-7	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
8	7-8	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
9	8-9	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
10	9-10	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
12	10-12	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
15	12-15	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
20	15-20	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
25	20-25	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
50	25-50	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
100	50-100	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%
	100 & >	0	63,400	0%	0%	0%	5	0	21,133	0%	0%	0%	20	0	21,133	0%	20	0	21,133	0%

Summary by SEC Rating Group			
SEC	Percent Satisfying DesARI of		
	5	10	20
< 4	33%	33%	33%
5 (4-6)	0%	0%	0%
6-9	0%	0%	0%
10 (9-12)	0%	0%	0%
12-20	0%	0%	0%
20 & >	0%	0%	0%
	33%	33%	33%
	100%	67%	33%

Stormwater Capacity Assessment

TABLE 2-1. SUMMARY FOR JOHNSTONS CREEK SWC 55

MAIN DRAIN or BRANCH No.	Name	OWNING AGENCY	SECTION	LENGTH	CROSS SECTION	HYDRAULIC CAPACITY	CATCHMENT AREA	RUNOFF	RATIO	ARI	Storm	Landuse	See	Note					
															INCOMING BRANCH SUB-BRANCH Land Feature	Nodes according to: HSD Region (Vol A2)	UPS	Section (m)	Rated Eq (m)
			TOTAL	10,723.7															
55	MAIN CHANNEL BRANCH 55A		A-B	321.87	1	15545	1625	0	OC	C	0.014	35.00	457.7	72.32	0.484	312	0.5-1	5	3,6
55	MAIN CHANNEL		B-C	51.21	1	15545	1625	0	OC	C	0.014	35.00	431.2	71.52	0.489	315	0.5-1	5	
55	MAIN CHANNEL BRANCH 55B		C-D	170.38	1	15545	1625	0	OC	C	0.014	56.00	425.7	71.17	0.787	491	2-3	5	
55	MAIN CHANNEL BRANCH 55C		D-E	276.75	1	12650	1625	0	OC	C	0.014	29.80	376.0	64.67	0.461	296	0.5-1	5	
55	MAIN CHANNEL BRANCH 55D		E-F	143.87	1	12650	1625	0	OC	C	0.014	29.80	365.7	65.75	0.453	290	0.5-1	5	
55	MAIN CHANNEL		F-G	110.03	1	11580	1625	0	OC	C	0.014	47.00	352.7	64.88	0.724	450	2-3	5	
55	MAIN CHANNEL BRANCH 55E		G-H	80.16	1	11885	2200	0	OC	C	0.014	51.60	350.8	65.81	0.784	487	2-3	5	
55	MAIN CHANNEL		H-J	59.44	1	8535	2185	0	OC	C	0.014	51.70	253.8	48.98	1.056	601	6-7	10	
55	MAIN CHANNEL Booth St		J-K	56.69	1				OT	C	0.014	51.70	252.3	49.02	1.055	600	6-7	10	4
55	MAIN CHANNEL BRANCH 55P		K-L	246.58	1	6170	2845	0	OC	C	0.014	51.90	231.9	44.88	1.156	647	8-9	10	
55	MAIN CHANNEL		L-M	121.31	1	5270	2970	0	OC	C	0.014	44.30	222.7	44.15	1.003	577	5-6	10	
55	MAIN CHANNEL BRANCH 55Q		M-N	99.06	1	2310	2985	0	CV	C	0.014	44.50	221.6	44.53	0.999	574	4-5	10	
55	MAIN CHANNEL		N-O	39.01	1	2310	2830	0	CV	C	0.014	40.80	110.2	22.16	1.841	951	25-50	10	
55	MAIN CHANNEL		O-P	46.63	1	2310	2590	0	CV	C	0.014	35.00	103.4	20.76	1.686	895	25-50	10	
55	MAIN CHANNEL Parramatta Rd		P-Q	58.52	1	2230	2590	0	CV	C	0.014	24.00	101.0	20.30	1.182	658	9-10	20	
55	MAIN CHANNEL BRANCH 55S		Q-R	377.34	1	2110	2235	0	CV	C	0.014	21.00	97.5	19.65	1.069	609	6-7	5	
55	MAIN CHANNEL Salisbury Rd		R-S	77.42	1	1550	1245	0	CV	C	0.014	7.18	24.0	5.40	1.330	732	12-15	5	
55	MAIN CHANNEL Cardigan St		S-T	65.84	1	1550	1245	0	CV	C	0.014	6.66	22.9	5.15	1.293	711	12-15	5	
55	MAIN CHANNEL Cardigan St		T-U	143.87	1	1295	1065	0	CV	C	0.014	4.21	22.5	5.06	0.832	498	3-4	5	
55	MAIN CHANNEL SE/railway Ave		U-V	181.97	1	1065	940	0	CV	C	0.014	3.80	18.1	4.07	0.934	546	4-5	5	
55	MAIN CHANNEL BRANCH 55U		V-A	51.95	1				Var	C	0.014	4.08	13.6	3.08	1.324	728	12-15	10	
55	MAIN CHANNEL		A-W	45.00	1				1150	P	0.013	5.43	0.1	0.02	#####	119918	100 &	10	
55A	FEDERAL PARK BRANCH		B-B1	17.69	1				CT	C	0.014	2.12	0.0				9-10	10	
55A	FEDERAL PARK BRANCH		B-B1	17.69	1				1350	P	0.013	2.48	17.4	3.88	1.186	661	9-10	10	
55A	FEDERAL PARK BRANCH		B1-	64.81	1	1360	1220	0	CV	C	0.014	2.12	0.0				9-10	10	
55A	FEDERAL PARK BRANCH		B1-	64.81	1	1350	1350	0	P	C	0.013	2.48	17.4	3.88	1.186	661	9-10	10	
55A	FEDERAL PARK BRANCH		B2	104.16	1	1360	1220	0	CV	C	0.014	2.12	15.5	3.44	0.616	390	1-2	10	

No.	MAIN DRAIN or BRANCH Name	OWNING PARTY	SECTION		LENGTH		CROSS SECTION				HYDRAULIC CAPACITY (Mannings)	CATCHMENT AREA (ha)	RUNOFF COEFFICIENT	RATIO	ARI	Storm Event Capacity (years)	Landuse Design ARI (years)	See Note
			Nodes according to: HSD Region UPS	Section (m)	Rated (m)	Eq (m)	No. off	Width (mm)	Ht (mm)	Dia. (mm)								
55B	LILLIE BRIDGE BRANCH		D-D3	D-D3	D-D3	12.12	0.00	0	CT	C	0.014	48.2	10.36			2-3	10	
55B	LILLIE BRIDGE B Harold Park		D3-D4	D3-D4	D3-D4	204.03	204.03	1	2440	925	0	3.46	9.74	0.355		225	0.5-1	10
55B	LILLIE BRIDGE B Harold Park		D4-D5	D4-D5	D4-D5	194.73	194.73	1	0	1350	P	5.92	7.64	0.774		478	2-3	10
55B	LILLIE BRIDGE B Wigram Rd		D5-D6	D5-D6	D5-D6	50.49	50.49	1	0	1200	P	4.33	5.47	0.791		488	2-3	5
55B	LILLIE BRIDGE BRANCH		D6-D7	D6-D7	D6-D7	82.84	82.84	1	0	1050	P	3.03	5.47	0.554		351	1-2	5
55B	LILLIE BRIDGE B Hegarty St		D7-D8	D7-D8	D7-D8	185.18	185.18	1	0	900	P	4.22	5.02	0.840		502	3-4	5
55B	LILLIE BRIDGE B John St		D8-D9	D8-D9	D8-D9	57.43	57.43	1	0	900	P	2.18	3.50	0.623		395	1-2	5
55B	LILLIE BRIDGE B Clare St		D9-D10	D9-D10	D9-D10	60.65	60.65	1	0	750	P	2.49	3.25	0.767		473	2-3	5
55B	LILLIE BRIDGE Bont Bridge Rd		D10-D11	D10-D11	D10-D11	27.94	27.94	1	0	750	P	2.03	3.25	0.625		396	1-2	10
55B	LILLIE BRIDGE BRANCH		D11-D12	D11-D12	D11-D12	65.32	65.32	1	0	750	P	1.65	2.17	0.760		469	2-3	5
55B	LILLIE BRIDGE BRANCH		D12-D13	D12-D13	D12-D13	70.27	70.27	1	595	840	O	1.23	1.81	0.677		429	1-2	5
55C	PIPER ST BRANCH		E-E14	E-E14	E-E14	61.99	61.99	1	0	600	P	0.45	1.17	0.388		246	0.5-1	10
55D	TAYLOR ST BRANCH		F-F15	F-F15	F-F15	103.09	103.09	1	1080	830	O	1.21	2.26	0.534		338	1-2	10
55E	ORPHAN SCHOOL CREEK BRANCH		H-H15	H-H15	H-H15	106.12	106.12	1	2820	1370	O	15.40	19.26	0.800		494	2-3	10
55E	ORPHAN SCHOOL CREEK BRANCH		H15-H16	H15-H16	H15-H16	13.36	13.36	1	2820	1370	O	17.20	18.49	0.930		544	4-5	10
55E	ORPHAN SCHOOL CREEK BRANCH		H16-H17	H16-H17	H16-H17	12.79	12.79	1	2820	1370	O	21.50	18.51	1.161		651	8-9	10
55E	ORPHAN SCHOOL CREEK BRANCH		H17-H18	H17-H18	H17-H18	60.35	60.35	1	2820	1370	O	17.20	18.53	0.928		543	4-5	10
55E	ORPHAN SCHOOL CREEK BRANCH		H18-H19	H18-H19	H18-H19	44.26	44.26	1	2285	1370	O	15.90	18.35	0.866		518	3-4	10
55E	ORPHAN SCHOOL CREEK BRANCH		H19-H20	H19-H20	H19-H20	75.32	75.32	1	2210	1370	O	13.10	18.35	0.714		441	2-3	10
55E	ORPHAN SCHOOL CREEK BRANCH		H20-H21	H20-H21	H20-H21	37.29	37.29	1	1905	1370	O	15.10	17.63	0.856		512	3-4	10
55E	ORPHAN SCHOOL CREEK BRANCH		H21-H22	H21-H22	H21-H22	40.18	40.18	1	1830	1370	O	15.30	17.68	0.865		517	3-4	10
55E	ORPHAN SCHOOL CREEK BRANCH		H22-H23	H22-H23	H22-H23	160.93	160.93	1	1525	1370	O	11.30	16.20	0.697		430	1-2	10
55E	ORPHAN SCHOOL CREEK BRANCH		H23-H24	H23-H24	H23-H24	170.99	170.99	1	1370	1220	O	8.30	14.92	0.556		352	1-2	10
55E	ORPHAN SCHOOL CREEK BRANCH		H24-H25	H24-H25	H24-H25	134.90	134.90	1	1370	1065	O	6.96	10.47	0.665		421	1-2	5
55E	ORPHAN SCHOOL CREEK BRANCH		H25-H26	H25-H26	H25-H26	20.34	20.34	1	1370	1065	O	6.96	9.83	0.708		449	2-3	5
55E	ORPHAN SCHOOL CREEK BRANCH		H26-H27	H26-H27	H26-H27	39.03	39.03	1	0	1350	P	7.19	9.83	0.731		451	2-3	5
55E	ORPHAN SCHOOL CREEK BRANCH		H27-H28	H27-H28	H27-H28	50.49	50.49	1	0	1350	P	3.94	4.86	0.811		485	2-3	5
55E	ORPHAN SCHOOL CREEK BRANCH		H28-H29	H28-H29	H28-H29	117.48	117.48	1	0	1050	P	3.84	4.86	0.791		488	2-3	5
55E	ORPHAN SCHOOL CREEK BRANCH		H29-H30	H29-H30	H29-H30	48.77	48.77	1	0	1050	P	5.24	2.64	1.983		1031	50-100	5
55E	ORPHAN SCHOOL CREEK BRANCH		H30-H31	H30-H31	H30-H31	78.64	78.64	1	0	1200	P	4.54	2.64	1.718		916	25-50	5
55E	ORPHAN SCHOOL CREEK BRANCH		H31-H32	H31-H32	H31-H32	42.92	42.92	1	0	1050	P	3.97	2.49	1.597		852	25-50	5
55E	ORPHAN SCHOOL CREEK BRANCH		H32-H33	H32-H33	H32-H33	24.59	24.59	1	0	900	P	2.64	2.49	1.062		605	6-7	5
55E	ORPHAN SCHOOL CREEK BRANCH		H33-H34	H33-H34	H33-H34	3.11	3.11	1	0	900	P	1.47	2.49	0.591		374	1-2	5

No.	MAIN DRAIN or BRANCH Name	INCOMING SUB-BRANCH Land Feature	Own If not Sydney	SECTION		LENGTH Section (m)	Rated Eq (m)	CROSS SECTION				HYDRAULIC CAPACITY Mann(mumecs)	CATCHMENT AREA (ha)	RUNOFF 5 yr ARI Peak Flow (mecs)	RATIO Capacity Peak = KdY.Fd	ARI Storm Event Capacity (years)	Land Use Design ARI (years)	See Note				
				NODES according to: HSD Region (Vol A2)	UPS			No. off	Width (mm)	Ht (mm)	Dia. Typ (mm)								Mtl	Capacit	5yr Peak	
55E	ORPHAN SCHOO	Sydney Uni		H33-H34	H33-H34	10.97	10.97	1	0	0	750	P	C	0.073	1.96	2.9	0.67	2.945	1496	100 &	5	
55E	ORPHAN SCHOO	Sydney Uni		H34-H35	H34-H35	103.20	103.20	1	0	0	750	P	C	0.073	1.96	2.9	0.67	2.945	1496	100 &	5	
55E	ORPHAN SCHOO	Sydney Uni		H35-H36	H35-H36	72.36	72.36	1	0	0	600	P	C	0.073	1.44	2.6	0.61	2.379	1209	100 &	5	
55E	ORPHAN SCHOOL CK		C	H36-H37	H36-H37	12.27	12.27	1	0	0	450	P	C	0.073	0.88	2.4	0.55	1.598	852	25-50	10	
55E	ORPHAN SCHOOL CREEK BRAN			H37-H38	H37-H38	68.60	68.60	1	0	0	450	P	C	0.073	0.49	1.3	0.31	1.584	845	25-50	10	
55E	ORPHAN SCHOOL CREEK BRAN			H38-H39	H38-H39	41.84	41.84	1	0	0	450	P	C	0.073	0.61	0.8	0.19	3.174	1613	100 &	5	
55E	ORPHAN SCHOOL CREEK BRAN			H39-H40	H39-H40	40.25	40.25	1	0	0	300	P	C	0.073	0.12	0.8	0.19	0.624	395	1-2	5	
55F	SCIENCE RD SUE	Sydney Uni		H24A-H24A	H24A-H24A	145.00	0.00	1	0	0	450	P	C	0.073	lot.AVA	2.8	0.63					5
55G	HOCKEY FIELD 5	Sydney Uni		H26-H45	H26-H45	191.91	191.91	1	915	915	0	B	C	0.073	2.07	16.5	3.57	0.579	367	1-2	5	
55G	HOCKEY FIELD 5	Sydney Uni		H45-H46	H45-H46	146.85	146.85	1	0	0	600	P	C	0.073	0.97	7.5	1.62	0.598	379	1-2	5	
55G	HOCKEY FIELD 5	Sydney Uni		H46-H47	H46-H47	105.14	105.14	1	0	0	525	P	C	0.073	0.62	4.6	1.01	0.615	390	1-2	5	
55G	HOCKEY FIELD 5	Sydney Uni		H47-H48	H47-H48	74.03	74.03	1	0	0	525	P	C	0.073	0.76	1.5	0.32	2.349	1193	100 &	5	
55H	PHYSICS SCHOOL	Sydney Uni		H26-H49	H26-H49	76.13	76.13	1	0	0	900	P	C	0.073	1.96	7.5	1.40	1.397	761	15-20	5	
55H	PHYSICS SCHOOL	Sydney Uni		H49-H50	H49-H50	40.20	40.20	1	0	0	900	P	C	0.073	1.87	7.5	1.40	1.333	733	12-15	5	
55H	PHYSICS SCHOOL	Sydney Uni		H50-H51	H50-H51	55.08	55.08	1	0	0	900	P	C	0.073	1.74	7.5	1.40	1.240	687	10-12	5	
55H	PHYSICS SCHOOL	Sydney Uni		H51-H52	H51-H52	49.16	49.16	1	0	0	900	P	C	0.073	2.25	7.5	1.40	1.604	856	25-50	5	
55H	PHYSICS SCHOOL	Sydney Uni		H52-H53	H52-H53	95.54	95.54	1	0	0	900	P	C	0.073	1.62	7.5	1.40	1.155	648	8-9	5	
55H	PHYSICS SCHOOL	Sydney Uni		H53-H54	H53-H54	41.96	41.96	1	0	0	900	P	C	0.073	1.65	7.5	1.40	1.176	656	9-10	5	
55H	PHYSICS SCHOOL	Sydney Uni		H54-H55	H54-H55	85.22	85.22	1	0	0	900	P	C	0.073	2.13	7.5	1.40	1.518	816	20-25	5	
55H	PHYSICS SCHOOL BRANCH 55J			H55-	H55-H55A	58.96	58.96	1	0	0	900	P	C	0.073	2.13	7.5	1.40	1.518	816	20-25	5	
55H	PHYSICS SCHOOL	Sydney Uni		H48	H55A-H48	36.42	36.42	1	0	0	900	P	C	0.073	2.13	2.6	0.60	3.534	1795	100 &	5	
55H	PHYSICS SCHOOL	Sydney Uni		H48-H48A	H48-H48A	56.30	56.30	1	0	0	375	P	C	0.073	0.32	2.1	0.49	0.640	406	1-2	5	
55J	SUB-BRANCH OFF PHYSICS SCH			H55A-H55A	H55A-H55A	36.00	36.00	1	0	0	450	P	C	0.073	0.34	4.8	0.80	0.421	267	0.5-1	5	
55E	ORPHAN SCHOOL CREEK SUB-B			H27A-	H27A-H28	15.59	15.59	1	0	0	1050	P	C	0.073	3.83	11.3	2.21	1.729	922	25-50	5	
55E	ORPHAN SCHOOL CREEK SUB-B			H28-H29	H28-H29	18.90	18.90	1	0	0	900	P	C	0.073	3.28	5.0	0.99	3.324	1689	100 &	5	
55E	ORPHAN SCHOOL CREEK SUB-B			H29-H30	H29-H30	81.30	81.30	1	0	0	900	P	C	0.073	3.28	0.9	0.18	17.845	9066	100 &	5	
55L	NEW MEDICAL SCHOOL RELIEF			H29-H29A	H29-H29A	94.55	94.55	1	0	0	600	P	C	0.073	0.62	0.3	0.07	9.155	4651	100 &	5	
55N	ST ANDREWS COLLEGE SUB-BRA			H33-H42	H33-H42	45.72	45.72	1	0	0	675	P	C	0.073	1.18	8.5	1.82	0.648	411	1-2	5	
55N	ST ANDREWS COLLEGE SUB-BRA			H42-	H42-H42A	36.58	36.58	1	0	0	675	P	C	0.073	1.27	7.5	1.63	0.778	480	2-3	5	
55N	ST ANDREWS COLLEGE SUB-BRA			H43	H42A-H43	39.62	39.62	1	0	0	600	P	C	0.073	1.21	4.5	1.07	1.128	633	7-8	5	
55N	ST ANDREWS COLLEGE SUB-BRA			H43-H44	H43-H44	41.33	41.33	1	0	0	600	P	C	0.073	1.66	4.5	1.07	1.548	826	20-25	5	

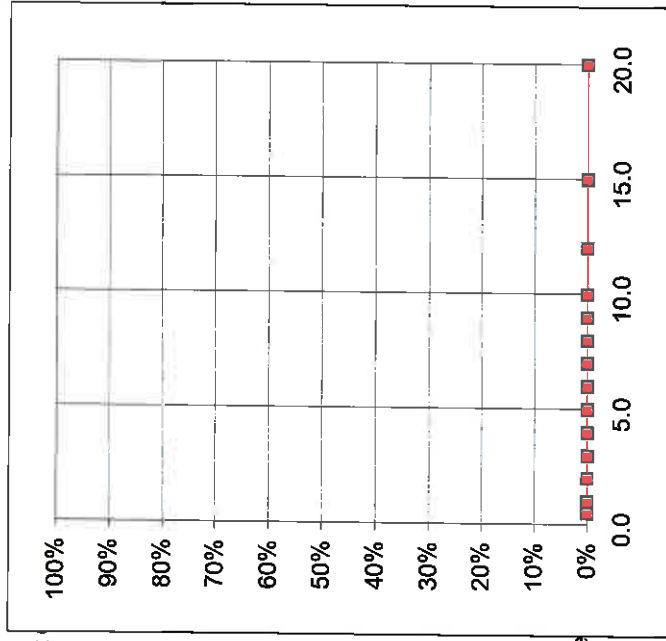
No.	MAIN DRAIN or BRANCH Name	INCOMING BRANCH SUB-BRANCH Land Feature	Own If not by other	SECTION		LENGTH Section (m)	Rate/Eq (m)	No. off	CROSS SECTION			HYDRAULIC CAPACITY Mannin(mums)	CATCHMENT AREA (ha)	RUNOFF 5 yr ARI Peak Flow (mums)	RATIO Capacity Peak = KdY.Fd	Ratio x ARI Storm Event Capacity (years)	See Design ARI Note (years)			
				NODES according to: HSD Region UPS	SECTION				Ht (mm)	Width (mm)	Dia. Typ (mm)							Capacity (mums)	Area (ha)	Flow (mums)
55P	GEHRIGS CREEK BRANCH		L-	L-56A	L-56A	15.53	15.53	1	1235	1215	0	CV	0.014	2.90	3.1	0.74	3.919	1991	100 & 10	
55P	GEHRIGS CREEK BRANCH		-L56	L56A-L56	L56A-L56	14.32	14.32	1	900	900	0	P	0.013	2.90	3.1	0.74	3.919	1991	100 & 10	
55P	GEHRIGS CREEK BRANCH		L56-L57	L56-L57	L56-L57	70.86	70.86	1	0	0	750	P	0.013	2.00	2.6	0.62	3.223	1637	100 & 10	
55P	GEHRIGS CREEK BRANCH		L57-L58	L57-L58	L57-L58	42.34	42.34	1	0	0	750	P	0.013	2.31	1.8	0.43	5.377	2732	100 & 10	
55Q	CARDIGAN ST BRANCH		N-Q66	N-Q66	N-Q66	158.80	158.80	1	1850	2285	0	CV	0.014	25.10	97.4	20.34	1.234	683	10-12	10
55Q	CARDIGAN ST B BRANCH 55R		Q66-Q103	Q66-N59	Q66-N59	15.00	15.00	1	1850	2285	0	CV	0.014	25.10	81.6	17.36	1.446	786	15-20	10
55Q	CARDIGAN ST B BRCH 55R(AMP)		Q103-N60	N59-N60	N59-N60	145.39	145.39	1	1850	1525	0	CV	0.014	14.80	52.1	11.17	1.325	728	12-15	10
55Q	CARDIGAN ST BH 55T (portion)		N60-N61	N60-N61	N60-N61	231.73	231.73	1	1850	1370	0	CV	0.014	15.50	49.9	10.85	1.429	777	15-20	10
55Q	CARDIGAN ST BH 55T (Balance)		N61-N62	N61-N62	N61-N62	109.07	109.07	1	2285	1370	0	CV	0.014	14.00	42.4	9.46	1.480	795	20-25	10
55Q	CARDIGAN ST B Cardigan Ln		N62-	N62-	N62-N62A	78.86	78.86	1	1905	1220	0	CV	0.014	11.90	33.1	7.32	1.626	867	25-50	10
55Q	CARDIGAN ST B Cardigan Ln		-N63	-N63	N62A-N63	126.92	126.92	1	1905	1220	0	CV	0.014	11.60	33.1	7.39	1.569	837	25-50	10
55Q	CARDIGAN ST B Sandbrook Ln		N63-N64	N63-N64	N63-N64	50.90	50.90	1	1600	1220	0	CV	0.014	11.20	31.5	7.06	1.585	845	25-50	10
55Q	CARDIGAN ST B In Rd/Pierce St		N64-N65	N64-N65	N64-N65	51.55	51.55	1	1600	1220	0	CV	0.014	11.90	31.5	7.06	1.685	899	25-50	10
55Q	CARDIGAN ST B Bedford St		N65-W	N65-N65A	N65-N65A	28.96	28.96	1	2515	1220	0	CV	0.014	7.38	30.6	6.84	1.079	615	6-7	10
55Q	CARDIGAN ST B Railway SRA		W-Z	N65A-Z	N65A-Z	35.97	35.97	1	1220	1590	0	B	0.014	5.18	26.2	5.82	0.890	532	3-4	10
55U	GLADSTONE ST BRANCH		Z-BU1	Z-VB1	Z-VB1	8.23	8.23	1	1370	915	0	B	0.014	4.49	26.2	5.82	0.771	476	2-3	10
55U	GLADSTONE ST BRANCH		Bu1-C	VB1-VC	VB1-VC	17.56	17.56	1	1525	1065	0	B	0.014	6.37	25.1	5.57	1.144	642	8-9	5
55U	GLADSTONE ST Trafalgar St		C-Cu1	VC-VC1	VC-VC1	45.35	45.35	1	1065	1065	0	B	0.014	5.49	23.2	5.12	1.072	611	6-7	5
55U	GLADSTONE ST BRANCH		Cu1-D	VC1-VD	VC1-VD	132.07	132.07	1	0	0	1350	P	0.013	5.43	23.2	5.12	1.060	604	6-7	5
55U	GLADSTONE ST Gladstone St		D-E	VD-VE	VD-VE	133.97	133.97	1	0	0	1200	P	0.013	4.16	17.2	3.72	1.117	631	7-8	10
55U	GLADSTONE ST Gladstone St		E-Eu1	VE-VE1	VE-VE1	36.56	36.56	1	0	0	1050	P	0.013	3.96					25-50	10
55U	GLADSTONE ST BRANCH		C	E-L	VE-VEL			1	0	0	525	P	0.013	1.11					25-50	10
55U	GLADSTONE ST BRANCH		C	E-L	VE-VEL			1	0	0	600	P	0.013	0.85	17.2	3.72	1.590	848	25-50	10
55U	GLADSTONE ST Gladstone St		Eu1-Eu2	VE1-VE2	VE1-VE2	119.74	119.74	1	0	0	1050	P	0.013	3.96					100 & 10	10
55U	GLADSTONE ST BRANCH		L-M	VE1-VE2	VE1-VE2	59.72	59.72	1	915	915	0	B	0.014	4.35	17.2	3.72	2.232	1134	100 & 10	10
55U	GLADSTONE ST BRANCH		Eu2-F	VE2-VF	VE2-VF			1	0	0	1050	P	0.013	2.85					9-10	10
55U	GLADSTONE ST BRANCH		M-F	VE1M-VF	VE1M-VF			1	0	0	900	P	0.013	1.60	17.2	3.72	1.195	666	9-10	10
55U	SUB-BRANCH OFF GLADSTONE			VD-VDA	VD-VDA	17.19	17.19	1	900	900	P	0.013	1.44	5.1	1.19	1.206	669	10-12	5	

MAIN DRAIN or BRANCH No.	INCOMING BRANCH SUB-BRANCH (Land Feature)	Own if not Synchronized	SECTION		LENGTH Section (m)	RATED Eq (m)	CROSS SECTION				HYDRAULIC CAPACITY (m <sup>3</sup> /s)	CATCHMENT AREA (ha)	RUNOFF (mm)	RATIO (Peak Flow / Peak Capacity)	Event Capacity (years)	Storm Design ARI (years)	Land Use Note			
			NODES according to: HSD Region UPS	SECTION			No. off	Width (mm)	Ht (mm)	Dia. (mm)								Typ. Mtl	Capacit Mannin	Peak Syr = kdy.Fd
55R	FOWLERS CREEK BRANCH		Q66-Q67	Q66-Q67	31.62		1	1065	915	0	B	C	0.014	4.71			15-20	10		
55W	FOWLERS CREEK BRANCH AMP		Q103-Q89	N59-Q89	28.61	30.12	1	0	0	1200	P	C	0.013	8.76	45.3	9.96	1.353	737	15-20	10
55R	FOWLERS CREEK BRANCH		Q67-Q68	Q67-Q68	132.97		1	0	0	1200	P	C	0.013	5.48				15-20	10	
55W	FOWLERS CREEK BRANCH AMP		Q89-Q90	Q89-Q90	134.39	133.68	1	0	0	1350	P	C	0.013	7.48	43.7	9.58	1.353	737	15-20	10
55R	FOWLERS CREEK BRANCH		Q68-Q69	Q68-Q69	105.21		1	0	0	1200	P	C	0.013	5.01				10-12	10	
55W	FOWLERS CREEK BRANCH AMP		Q90-Q91	Q90-Q91	147.19	126.20	1	0	0	1500	P	C	0.013	5.48	39.2	8.53	1.230	682	10-12	10
55T	KINGSTON RD RELIEF DRAIN	C	N62-T71	N61-T71	39.87	0.00	1	0	0	750	P	C	0.013	1.24				3-4	10	
55T	KINGSTON RD BRANCH		N62-T71	N62-T71	39.87	39.87	1	0	0	840	C	C	0.014	1.91	15.3	3.57	0.883	528	3-4	10
55S	STANMORE BRANCH AMP		R-R70	R-R70	72.83		1	1065	1065	0	CV	C	0.014	3.51				2-3	10	
55S	STANMORE BRA Salisbury Rd		R-R70	R-R70	80.62	76.73	1	1295	1065	0	CV	C	0.014	5.68	57.0	11.50	0.799	495	2-3	10
55S	STANMORE BRANCH AMP		R70-R71	R70-R71	261.52	261.52	1	1065	1065	0	CV	C	0.014	3.21				2-3	10	
55S	STANMORE BRA Salisbury Rd		R70-R71	R70-R71	261.52	261.52	1	1295	1065	0	CV	C	0.014	5.69	54.5	11.09	0.802	496	2-3	10
55S	STANMORE BRANCH AMP		R71-R72	R71-R72	52.07	52.07	1	0	0	900	P	C	0.013	2.09				2-3	5	
55S	STANMORE BRA Salisbury Rd		R71-R72	R71-R72	52.07	52.07	1	1295	1065	0	CV	C	0.014	5.55	47.2	9.90	0.772	477	2-3	5
55S	STANMORE BRANCH AMP		R72-R73	R72-R73	61.39	61.39	1	0	0	900	P	C	0.013	2.09				2-3	5	
55S	STANMORE BRA Salisbury Rd		R72-R73	R72-R73	61.39	61.39	1	1040	965	0	CV	C	0.014	4.30	37.3	7.88	0.810	485	2-3	5
55S	STANMORE BRANCH AMP		R73-R74	R73-R74	188.29	188.29	1	0	0	750	P	C	0.013	1.49				2-3	5	
55S	STANMORE BRA Salisbury Rd		R73-R74	R73-R74	188.29	188.29	1	1040	865	0	CV	C	0.014	3.73	31.5	6.73	0.775	479	2-3	5
55S	STANMORE BRANCH AMP		R74-R75	R74-R75	97.97	97.97	1	0	0	900	P	C	0.013	1.80				3-4	10	
55S	STANMORE BRA JY/Perival Rd		R74-R75	R74-R75	100.45	99.21	1	1040	865	0	CV	C	0.014	3.73	30.4	6.63	0.834	499	3-4	10
55S	STANMORE BRA Rd/Douglas St		R75-R76	R75-R76	14.75	14.75	1	1040	865	0	CV	C	0.014	3.73	21.7	4.69	0.795	491	2-3	10
55S	STANMORE BRA Gordon Cres		R76-R77	R76-R77	51.00	51.00	1	1040	865	0	CV	C	0.014	3.73	19.1	4.13	0.903	528	3-4	10
55S	STANMORE BRA Gordon Cres		R77-R78	R77-R78	159.12	159.12	1	1040	865	0	CV	C	0.014	3.46	19.1	4.13	0.838	501	3-4	5
55S	STANMORE BRA Gordon Cres		R78-R79	R78-R79	69.88	69.88	1	0	0	600	P	C	0.013	1.17	3.7	0.80	1.462	786	15-20	5
55U	GLADSTONE ST Railway		A-B	VA-VB	87.45	87.45	1	0	0	1050	P	C	0.013	4.03	13.5	3.06	1.317	724	12-15	10
55U	GLADSTONE ST BRANCH (D/S PC)		B-Bu1	VB-VB1	39.40	39.40	1	0	0	900	P	C	0.013	1.89	4.0	0.91	2.085	1084	50-100	10

- Section lengths and cross-sectional dimensions from Region's Node Diagram.
- Capacity values outlined with double lines include overbank capacity.
- A-B has overbank capacity downstream of the footbridge which would raise its event capacity to 20-25 Years ARI.
- J-K includes an arched culvert at Booth St.
- Taylor St Branch actually varies from 1020 x 685 to 1140 x 975. Dimensions shown are those midreach where runoff from Taylor St enters.
- Catchment areas based on the Region's Johnston's Creek Flood Study, 1995 and Hydraulic Survey Sheets. The total calculated by the UPS GIS is 460.0 ha.

*Stormwater Capacity Assessment*

(a) PERCENT OF DRAINAGE SYSTEM SATISFYING ARI STORM EVENT



ARI Storm Event - years

(b) GROUPED BY LANDUSE BASED DESIGN ARI SPECIFICATION OF

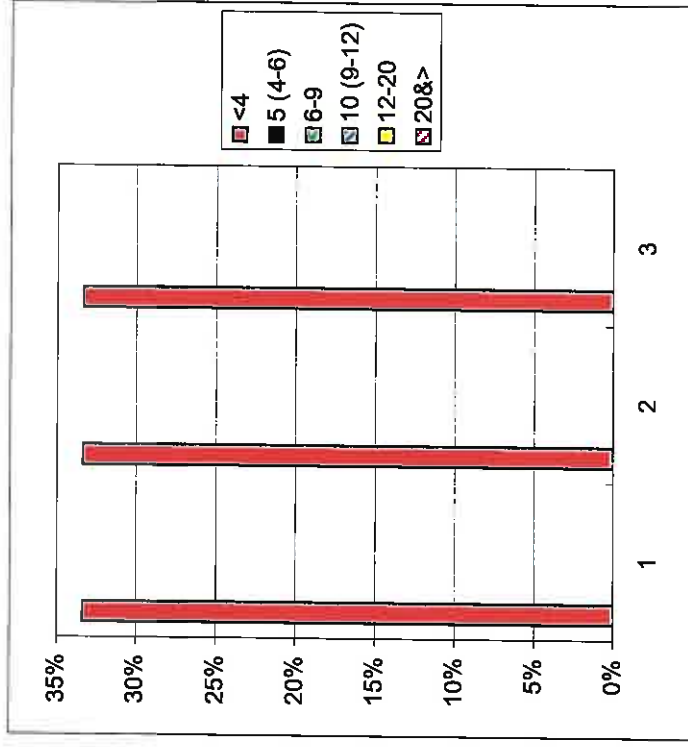


FIGURE 2-2. PERFORMANCE SUMMARY FOR JOHNSTONS CREEK SWC 55

TABLE 2-3A. HYDRAULIC CAPACITY CALCULATION  
JOHNSTONS CREEK SWC 55

Stormwater Capacity Assessment

SECTION UPS	Cross Section Type Shape Form.	Top Width (m)	Base Width Slope (m) kH or D	Centre Height (m)	Flow- line HdF (m)	Wall Trap Circ Only Height Slope kDV depth radius Y Hor d r (m) kVer (m)	Xsect. Area Ado (m <sup>2</sup> )	Wet. Perim. Pdo (m)	Hydr. Rad. Rdo (m)	Section Slope (Grade) (%)	Manning Rough ndo	No. Off	Capacity as built Qdo (cumecs)	Previous Analysis (if relevant) Mc4Fillwraith, n Capacity (cumecs)	Overbank Ratio Q/Qdo	Capacity referred to Sheet A: (cumecs)	See Note
A-B	OC Trap.Sp Open	15.545	15	1.360		5	17.16	16.98	1.01	0.0803%	0.014	1	35.00	0.013	36.05	35.00	2.3
B-C	ditto						17.16	16.98	1.01	0.0803%	0.014	1	35.00	0.013	36.05	35.00	2
C-D	ditto			1.360			17.16	16.98	1.01	0.0800%	0.014	1	34.90	0.013	35.79	1.60	56.00
D-E	OC Trap.Sp Open	12.650	15	1.360		5	14.53	14.23	1.02	0.0800%	0.014	1	29.80	0.013	30.81	29.80	2
E-F	ditto						14.53	14.23	1.02	0.0800%	0.014	1	29.80	0.013	30.81	29.80	2
F-G	OC Trap.Sp Open	11.580	15	1.360		5	13.49	13.22	1.02	0.0800%	0.014	1	27.60	0.013	24.04	1.72	47.00
G-H	OC Trap.Sp Open	11.885	15	1.935		5	20.34	14.47	1.41	0.0800%	0.014	1	51.60	0.013	55.93	51.60	2
H-J	OC Special Open	8.535	15	2.185		Var.	14.81	11.37	1.30	0.168%	0.014	1	51.70	0.013	53.52	51.70	
J-K	Transiti = Min									0.168%	0.014		51.70	0.013	54.51	51.70	
K-L	OC Trap.Sp Open	6.170	15	2.845		Var.	14.11	10.02	1.41	0.168%	0.014	1	51.90	0.013	54.31	51.90	
L-M	OC Special Open	5.270	15	2.970		Var.	12.51	9.42	1.33	0.168%	0.014	1	44.30	0.013	47.91	44.30	
M-N	CV Trap.Splowlin	2.310	1.390	Circ 2.985	2.925	Var.	5.07	6.77	0.75	2.220%	0.014	1	44.50	0.013	41.06	44.50	1*
N-O	ditto	2.310	1.390	Circ 2.830	2.775	Var.	4.73	6.48	0.73	2.220%	0.014	1	40.80	0.013	36.81	40.80	1*
O-P	ditto	2.310	1.390	Circ 2.590	2.540	Var.	4.18	6.00	0.70	2.220%	0.014	1	35.00	0.013	30.58	35.00	1*
P-Q	ditto	2.230	1.370	Circ 2.590	2.540	Var.	4.10	5.97	0.69	1.110%	0.014	1	24.00	0.013	21.15	24.00	1*
Q-R	CV Trap.Sp Open	2.110	1.370	Circ 2.235		Var.	3.45	5.39	0.64	1.315%	0.014	1	21.00	0.013	22.43	21.00	
R-S	CV Fillet.Relowlin	1.550	10	1.245	1.220		1.80	3.83	0.47	0.847%	0.014	1	7.18	0.013	6.65	7.18	1*
S-T	ditto						1.80	3.83	0.47	0.730%	0.014	1	6.66	0.013	6.15	6.66	1*
T-U	CV Fillet.Relowlin	1.295	10	1.065	1.045		1.28	3.24	0.40	0.730%	0.014	1	4.21	0.013	4.39	4.21	1*
U-V	ditto	1.065	10	0.940	0.920		0.92	2.78	0.33	1.455%	0.014	1	3.80	0.013	3.37	3.80	1*
V-VA	Variablelowline						1.08	3.68	0.29	2.220%	0.014		0.014	4.08	4.08	1(b)	
VA-W	Circular Full			1.170			1.08	3.68	0.29	2.220%	0.013	1	5.43	0.014	4.08	5.43	
B-B1	CT Transiti = U/s						1.26	3.03	0.42	0.180%	0.014	1	2.12	0.013	2.23	2.12	
B-B1	P Circular Full			1.370			1.47	4.30	0.34	0.200%	0.013	1	2.48	0.00	0.00	2.48	
B1-B1A	CV Trap.Cirwlin	1.360	1.050	Circ 1.220	1.195	5	1.26	3.03	0.42	0.180%	0.014	1	2.12	0.013	2.23	2.12	
B1-B1A	P Circular Full			1.370			1.47	4.30	0.34	0.200%	0.013	1	2.48	0.00	0.00	2.48	
B1A-B2	CV Trap.Cirwlin	1.360	1.050	Circ 1.220	1.195	5	1.26	3.03	0.42	0.180%	0.014	1	2.12	0.013	2.23	2.12	



SECTION UPS	Cross Section Type Shape Form.	Top Width (m)	Base Width Slope (m) kH or D	Centre Height (m)	Flow- line HdF (m)	Wall Trap Circ Only Height Slope kdv Hor d r	Xsect. Area Ado (mu2)	Wet. Hydr. Perim. Rad. Pdo Rdo (m)	Section Slope (Grade) (%)	Manning Rough ndo	No. Off	Capacity as built Qdo (cumecs)	Capacity previous Analysis (cumecs)	Overban Ratio Q/Qdo	Capacity referred to Sheet A: (cumecs)	See Notes
D-03	CT															
D3-D4	CV Fillet.Triowlin	2.440	15 0.925 0.905	1.370	5		1.95 3.80 0.51	0.152%	0.014	1	3.46	0.013	0.00		0.00	
D4-D5	P Circular Full						1.47 4.30 0.34	1.136%	0.013	1	5.92	0.013	2.79		3.46	1*
D5-D6	P								0.013			0.013	5.93		5.92	
D6-D7	P								0.013			0.013	4.33		4.33	
D7-D8	P								0.013			0.013	3.03		3.03	
D8-D9	P								0.013			0.013	4.22		4.22	
D9-D10	P								0.013			0.013	2.18		2.18	
D10-D11	P								0.013			0.013	2.49		2.49	
D11-D12	P								0.013			0.013	2.03		2.03	
D12-D13	OV								0.013			0.013	1.65		1.65	
E-E14	P Circular Full			0.610			0.29 1.92 0.15	0.500%	0.013	1	0.45	0.013	0.46		0.45	
F-F15	CV Trap.CirOpen	1.080	0.890	0.830	0.815	5	0.455	0.68 2.17 0.31	0.293%	0.014	1	1.21	0.013	0.74	1.21	4
H-H15	CV Trapezlowlin	2.820	2.665	15 1.370 1.345	1.280	16.5	3.56 5.19 0.69	0.606%	0.014	1	15.40	0.013	12.46		15.40	1*
H15-H16	CV ditto						3.56 5.19 0.69	0.758%	0.014	1	17.20	0.013	13.96		17.20	1*
H16-H17	CV ditto						3.56 5.19 0.69	1.183%	0.014	1	21.50	0.013	17.36		21.50	1*
H17-H18	CV ditto						3.56 5.19 0.69	0.758%	0.014	1	17.20	0.013	13.96		17.20	1*
H18-H19	CV Trapezlowlin	2.285	2.135	15 1.370 1.345	1.300	17.5	2.89 4.69 0.62	1.137%	0.014	1	15.90	0.013	12.97		15.90	1*
H19-H20	CV Fillet.Sp.:owlin	2.210	15 1.370 1.345				2.50 4.36 0.57	1.137%	0.014	1	13.10	0.013	12.77		13.10	
H20-H21	CV Fillet.Sp.:owlin	1.905	15 1.370 1.345				2.35 4.32 0.54	1.820%	0.014	1	15.10	0.013	12.74		15.10	
H21-H22	B Rectan:owlin	1.850	10 1.370 1.345	1.280			2.38 4.35 0.55	1.820%	0.014	1	15.30	0.013	0.00		15.30	
H22-H23	B Fillet.Retowlin	1.525	15 1.370 1.345				1.98 4.10 0.48	1.667%	0.014	1	11.30	0.013	10.59		11.30	
H23-H24	B Fillet.Retowlin	1.370	15 1.220 1.195				1.58 3.65 0.43	1.667%	0.014	1	8.30	0.013	7.63		8.30	
H24-H24A	CV Fillet.Retowlin	1.370	15 1.065 1.045				1.37 3.35 0.41	1.667%	0.014	1	6.96	0.013	6.29		6.96	
H24A-H25	CV Fillet.Retowlin	1.370	15 1.065 1.045				1.37 3.35 0.41	1.667%	0.014	1	6.96	0.013	6.29		6.96	
H25-H26	P								0.013			0.013	7.19		7.19	
H26-H27	P								0.013			0.013	3.94		3.94	
H27-H27A	P								0.013			0.013	3.84		3.84	
H27A-H27B	P								0.013			0.013	5.24		5.24	
H27B-H27C	P								0.013			0.013	4.54		4.54	
H27C-H27D	P								0.013			0.013	3.97		3.97	
H27D-H27E	P								0.013			0.013	2.64		2.64	
H27E-H33	P								0.013			0.013	1.47		1.47	

SECTION UPS	Cross Section Type Shape Form	Top Width (m)	Base		Centre Height (m)	Flow- line HdF (m)	Wall		Xsect. Area Addo (mu2)	Wet. Hydr. Perim. Rad. PDD Rdo (m)	Section Slope (Grade) (%)	Manning Rough n	No. Off	Capacity as built Qdo (cumecs)	Previous Analysis [if relevant] McqFillwraith, n Capacity (cumecs)	Overbar Ratio Q/Qdo	Capacity referred to Sheet A: (cumecs)	See Note	
			Width kD or Dia. (m)	Depth kH or D (m)			Height kV (m)	Height kV (m)											
H33-H34	P										0.013			0.013	1.96		1.96		
H34-H35	P										0.013			0.013	1.96		1.96		
H35-H36	P										0.013			0.013	1.44		1.44		
H36-H37	P										0.013			0.013	0.88		0.88		
H37-H38	P										0.013			0.013	0.49		0.49		
H38-H39	P										0.013			0.013	0.61		0.61		
H39-H40	P										0.013			0.013	0.12		0.12		
H24A-H24A	P										0.013				0.00		0.00		
H26-H45	B Trap.Cir/flow/ln	0.915	0.890	Circ 0.915	0.895		5	0.305	0.55	2.04	0.27	1.590%	0.014	1	2.07	0.013	2.02	2.07	1*
H45-H46	P										0.013				0.013	0.97	0.97	0.97	
H46-H47	P										0.013				0.013	0.62	0.62	0.62	
H47-H48	P										0.013				0.013	0.76	0.76	0.76	
H26-H49	P										0.013				0.013	1.96	1.96	1.96	
H49-H50	P										0.013				0.013	1.87	1.87	1.87	
H50-H51	P										0.013				0.013	1.74	1.74	1.74	
H51-H52	P										0.013				0.013	2.25	2.25	2.25	
H52-H53	P										0.013				0.013	1.62	1.62	1.62	
H53-H54	P										0.013				0.013	1.65	1.65	1.65	
H54-H55	P										0.013				0.013	2.13	2.13	2.13	
H55-H55A	P										0.013				0.013	2.13	2.13	2.13	
H55A-H48	P										0.013				0.013	2.13	2.13	2.13	
H48-H48A	P Circular Full				0.380				0.11	1.19	0.10	3.000%	0.013	1	0.32	0.00	0.00	0.32	
H55A-H55A	P Circular Full				0.455				0.16	1.43	0.11	1.316%	0.013	1	0.34	0.00	0.00	0.34	
H27A-H28	P Circular Full				1.065				0.89	3.35	0.27	1.820%	0.013	1	3.83	0.00	0.00	3.83	
H28-H29	P Circular Full				0.915				0.66	2.87	0.23	3.000%	0.013	1	3.28	0.00	0.00	3.28	
H29-H30	P Circular Full				0.915				0.66	2.87	0.23	3.000%	0.013	1	3.28	0.00	0.00	3.28	
H29-H29A	P										0.013				0.013	0.62	0.62	0.62	
H33-H42	P										0.013				0.013	1.18	1.18	1.18	
H42-H42A	P Circular Full				0.685				0.37	2.15	0.17	2.100%	0.013	1	1.27	0.00	0.00	1.27	

SECTION UPS	Cross Section Type Shape Form.	Top Width B (m)	Base Width b (m)	Centre Height H or Dia. (m)	Flow- line HdF (m)	Wall Trap Circ Only radius r (m)	Xsect. Area Add (mu2)	Wet. Perim. P (m)	Hydr. Rad. Rdo (m)	Section Slope (Grade) (%)	Manning Rough n	No. Off	Capacity as built Qdo (cumecs)	Previous Analysis [ If relevant ] Capacity Qdo (cumecs)	Overban Ratio Q/Qdo	Capacity referred to Sheet A: (cumecs)	See Note
H42A-H43	P Circular Full			0.610			0.29	1.92	0.15	3.570%	0.013	1	1.21	0.013	1.21	1.21	
H43-H44	P										0.013			0.013	1.66	1.66	
L-156A	CV Trap.Cirrowlin	1.235	0.890	Circ 1.215	1.190	5	1.11	2.93	0.38	0.488%	0.014	1	2.90	0.013	2.68	2.90	1*
L56A-L56	P Circular =Dis										0.013		2.90		0.00	2.90	
L56-L57	P Circular Full			0.760			0.45	2.39	0.19	3.000%	0.013	1	2.00		0.00	2.00	
L57-L58	P Circular Full			0.760			0.45	2.39	0.19	4.000%	0.013	1	2.31		0.00	2.31	
N-Q66	CV Rectant. Open	1.830		16 2.285	2.230		4.13	6.29	0.66	1.268%	0.014	1	25.10	0.013	23.02	25.10	1*
Q66-N59	CV ditto						4.13	6.29	0.66	1.268%	0.014	1	25.10	0.013	23.02	25.10	1*
N59-N60	CV ditto	1.830		16 1.525	1.495	1.470	2.69	4.71	0.57	1.268%	0.014	1	14.80	0.013	13.31	14.80	1*
N60-N61	CV ditto	1.830		16 1.370	1.345	1.315	2.41	4.41	0.55	1.820%	0.014	1	15.50	0.013	13.68	15.50	1*
N61-N62	CV ditto	2.285		16 1.370	1.345	1.300	2.99	4.84	0.62	0.813%	0.014	1	14.00	0.013	12.04	14.00	1*
N62-N62A	CV ditto	1.905		16 1.220	1.195	1.160	2.22	4.18	0.53	1.316%	0.014	1	11.90	0.013	10.31	11.90	1*
N62A-N63	CV ditto						2.22	4.18	0.53	1.250%	0.014	1	11.60	0.013	10.31	11.60	1*
N63-N64	CV ditto	1.600		16 1.220	1.195	1.170	1.87	3.89	0.48	1.855%	0.014	1	11.20	0.013	10.11	11.20	1*
N64-N65	CV ditto						1.87	3.89	0.48	2.085%	0.014	1	11.90	0.013	10.73	11.90	1*
N65-N65A	CV ditto	2.515		16 1.220	1.195	1.140	2.90	4.75	0.61	0.244%	0.014	1	7.38	0.013	6.13	7.38	1*
N65A-Z	B				1.220						0.014			0.013	5.58	5.18	
Z-VB1	B Rectant. lowlin	1.370		10 0.915	0.895	0.845	1.18	3.03	0.39	1.000%	0.014	1	4.49	0.013	3.98	4.49	1*
VB1-VC	B Rectant. lowlin	1.525		10 1.065	1.045	0.990	1.54	3.47	0.44	1.000%	0.014	1	6.37	0.013	5.61	6.37	1*
VC-VC1	B Rectant. lowlin	1.065		10 1.065	1.045	1.010	1.08	3.05	0.36	2.000%	0.014	1	5.49	0.013	4.93	5.49	1*
VC1-VD	P										0.013			0.013	5.43	5.43	
VD-VE	P										0.013			0.013	4.16	4.16	
VE-VE1	P										0.013			0.013	3.96	3.96	
VE-VEL	P										0.013			0.013	1.11	1.11	
VE-VEL	P										0.013			0.013	0.85	0.85	
VE1-VE2	P										0.013			0.013	3.96	3.96	
VEL-VEM	B Rectant. lowlin	0.915		10 0.915	0.895	0.870	0.80	2.62	0.30	2.840%	0.014	1	4.35	0.013	3.95	4.35	1*
VE2-VF	P										0.013			0.013	2.85	2.85	
VEM-VF	P										0.013			0.013	1.60	1.60	
VD-VDA	P Circular Full			0.915			0.66	2.87	0.23	0.580%	0.013	1	1.44		0.00	1.44	

SECTION UPS	Cross Section Type Shape Form.	Top Width B (m)	Base Width b (m)	Centre Height H or Dia. (m)	Flow- line HdF (m)	Wall Height kV (m)	Trap depth d (m)	Circ radius r (m)	Xsect. Area A00 (mu2)	Wet. Perim. P00 (m)	Hydr. Rad. R00 (m)	Section Slope (Grade) (%)	Manning Rough n00	No. Off	Capacity as built Q00 (cumecs)	Previous Capacity n (cumecs)	Analysis Ratio Q/Q00	Overbank Capacity to Sheet A: (cumecs)	See Note
Q66-Q67	B Fillet.Relowlin	1.065	10	0.915	0.895				0.89	2.73	0.33	2.410%	0.014	1	4.71	0.013	4.45	4.71	1*
N59-Q89	P												0.013			0.013	8.76	8.76	
Q67-Q68	P												0.013			0.013	5.48	5.48	
Q89-Q90	P												0.013			0.013	7.48	7.48	
Q68-Q69	P												0.013			0.013	5.01	5.01	
Q90-Q91	P												0.013			0.013	5.48	5.48	
N61-T71	P Circular Full			0.760					0.45	2.39	0.19	1.153%	0.013	1	1.24		0.00	1.24	
N62-T71	C Circular Full			0.840					0.55	2.64	0.21	1.856%	0.014	1	1.91	0.013	2.04	1.91	
R-R70	CV Fillet.Triowlin	1.065	15	1.065	1.045				0.86	2.73	0.32	1.516%	0.014	1	3.51	0.013	3.65	3.51	
R-R70	CV Fillet.ReOpen	1.295	10	1.065			5		1.31	3.28	0.40	1.265%	0.014	1	5.68	0.013	4.98	5.68	
R70-R71	CV Fillet.Triowlin	1.065	15	1.065	1.045				0.86	2.73	0.32	1.266%	0.014	1	3.21	0.013	3.33	3.21	
R70-R71	CV Fillet.ReOpen	1.295	10	1.065			5		1.31	3.28	0.40	1.266%	0.014	1	5.69	0.013	4.98	5.69	
R71-R72	P Circular Full			0.915					0.66	2.87	0.23	1.220%	0.013	1	2.09	0.013	2.08	2.09	
R71-R72	CV Fillet.Relowlin	1.295	10	1.065	1.045				1.28	3.24	0.40	1.268%	0.014	1	5.55	0.013	4.98	5.55	
R72-R73	P Circular Full			0.915					0.66	2.87	0.23	1.220%	0.013	1	2.09	0.013	2.08	2.09	
R72-R73	CV Fillet.Relowlin	1.040	10	0.965	0.945				0.93	2.81	0.33	1.854%	0.014	1	4.30	0.013	4.01	4.30	
R73-R74	P Circular Full			0.760					0.45	2.39	0.19	1.667%	0.013	1	1.49	0.013	1.50	1.49	
R73-R74	CV Fillet.Relowlin	1.040	10	0.865	0.850				0.83	2.62	0.32	1.854%	0.014	1	3.73	0.013	3.36	3.73	
R74-R75	P Circular Full			0.915					0.66	2.87	0.23	0.910%	0.013	1	1.80	0.013	1.80	1.80	
R74-R75	CV Fillet.Relowlin	1.040	10	0.865	0.850				0.83	2.62	0.32	1.854%	0.014	1	3.73	0.013	3.36	3.73	
R75-R76	CV ditto								0.83	2.62	0.32	1.854%	0.014	1	3.73	0.013	3.36	3.73	
R76-R77	CV ditto								0.83	2.62	0.32	1.854%	0.014	1	3.73	0.013	3.36	3.73	
R77-R78	CV ditto								0.83	2.62	0.32	1.590%	0.014	1	3.46	0.013	3.39	3.46	
R78-R79	P Circular Full			0.610					0.29	1.92	0.15	3.333%	0.013	1	1.17	0.013	1.17	1.17	
VA-VB	P												0.013			0.013	4.03	4.03	
VB-VB1	P												0.013			0.013	1.89	1.89	

1. Previous calculation by McIlwraith unless noted as: (a) Work plan or similar; (b) A flow schedule. \* denotes that McIlwraith treated as a closed section while others are
2. H, as if section did not have a low flow channel, for use in standard formula. Area and extra wetted perimeter for this less same for the fillets are added/subtracted.
3. Capacity of the portion of A-B downstream of footbridge is 3.1 times that of the formal channel which would raise its event capacity to 20-25 years ARI.
4. The capacity for the Taylor St Branch, which varies continuously, ranges from 0.88 cumecs to 1.56 cumecs. The value shown is that at midreach at Taylor St.

**TABLE 2-4. CO-EFFICIENT OF RUNOFF (COR), PEAK FLOW CALCULATION  
JOHNSTONS CREEK SWC 55**

*Stormwater Capacity Assessment*

SECTION	RUNOFF	CATCHMENT	ToC	INTENSITY	Equin	CorRnds	Open Space			Commercial			Industrial			Special Uses			Residential			See							
							PeakFlow	AREA	km5 =	%	%	%	%	%	%	%	%	%	%	%	%		%	%	%	%	%	%	
UPS	5 yr AR	Cum.	(ha)	(mm/hr)	(mm/hr)	(mm/hr)	%	%	%	%	%	%	%	%	%	%	%	%	%	%	%	%							
A-B	72.32	457.71	26.50	30.3	77.0	63%	0.74	0.59	80%	5%	0.57	15%	40%	0.67	5%	28	69%	0.76	5%	28	69%	0.76	Nil	edu/ha					
B-C	71.52	431.21	5.50	27.6	80.6	64%	0.74	0.72	10%	5%	0.57	40%	55%	0.72	20%	26	66%	0.75	30%	28	69%	0.76							
C-D	71.17	425.71	49.73	27.2	81.2	64%	0.74	0.72	30%	5%	0.57	40%	75%	0.77	30%	40	81%	0.79											
D-E	64.67	375.98	10.30	25.8	83.3	65%	0.74	0.72																					
E-F	65.75	365.68	13.00	23.6	87.0	65%	0.74	0.72																					
F-G	64.88	352.68	1.90	22.4	89.2	64%	0.74	0.72																					
G-H	65.81	350.78	96.94	21.5	90.9	65%	0.74	0.72																					
H-J	48.98	253.84	1.50	21.0	92.0	69%	0.76	0.72																					
J-K	49.02	252.34	20.40	20.7	92.6	69%	0.76	0.79	2%	10%	0.58	7%	90%	0.82	50%	85%	0.80	28%	75%	0.77	9%	35	77%	0.78	4%	40	81%	0.79	
K-L	44.88	231.94	9.20	20.7	92.6	68%	0.75	0.78	3%	5%	0.57	30%	85%	0.80	25%	70%	0.76	21%	35	77%	0.78	21%	35	77%	0.78	21%	40	81%	0.79
L-M	44.15	222.74	1.10	19.6	95.0	67%	0.75	0.78																					
M-N	44.53	221.64	111.42	19.0	96.3	67%	0.75	0.78																					
N-O	22.16	110.22	6.80	18.8	96.8	66%	0.75	0.78																					
O-P	20.76	103.42	2.40	18.7	96.9	65%	0.75	0.78																					
P-Q	20.30	101.02	3.50	18.6	97.2	65%	0.74	0.77																					
Q-R	19.65	97.52	73.52	18.5	97.6	65%	0.74	0.77																					
R-S	5.40	24.00	1.10	15.0	107.1	69%	0.76	0.75																					
S-T	5.15	22.90	0.40	15.0	107.1	69%	0.76	0.75																					
T-U	5.06	22.50	4.40	15.0	107.1	69%	0.76	0.76																					
U-V	4.07	18.10	4.50	15.0	107.1	69%	0.76	0.74																					
V-W	3.08	13.60	13.50	15.0	107.1	71%	0.76	0.74																					
VA-W	0.02	0.10	0.10	15.0	107.1	75%	0.77	0.77																					
B-B1				15.0																									
B-B1	3.88	17.40		15.0	107.1	67%	0.75	0.77																					
B1-B1A				15.0				0.77																					
B1-B1A	3.88	17.40	1.90	15.0	107.1	67%	0.75	0.77																					
B1A-B2	3.44	15.50	15.50	15.0	107.1	66%	0.75	0.75																					

SECTION	RUNOFF	CATCHMENT AREA	ToC	INTENSITY	EQUIV	CoR	LAND ZONING		See										
							PeakFlow	5 yr AR		Special Uses		Residential		Note					
										% Area	% Area	% Area	% Area						
D-D3	10.36	48.23	3.40	15.2	106.5	59%	0.73	0.64	15%	30%	0.64								
D3-D4	9.74	44.83	10.70	15.1	106.8	61%	0.73	0.67	75%	30%	0.64								
D4-D5	7.64	34.13	9.70	15.0	107.1	68%	0.75	0.75	3%	60%	0.73	29%	32	74%	0.77	###	41	82%	0.79
D5-D6	5.47	24.43	15.0		107.1	68%	0.75	0.75											
D6-D7	5.47	24.43	2.00	15.0	107.1	68%	0.75	0.75											
D7-D8	5.02	22.43	6.80	15.0	107.1	68%	0.75	0.75											
D8-D9	3.50	15.63	1.14	15.0	107.1	68%	0.75	0.75											
D9-D10	3.25	14.49	15.0		107.1	68%	0.75	0.75											
D10-D11	3.25	14.49	4.80	15.0	107.1	68%	0.75	0.75											
D11-D12	2.17	9.69	1.60	15.0	107.1	68%	0.75	0.75											
D12-D13	1.81	8.09	8.09	15.0	107.1	68%	0.75	0.75											
E-E14	1.17	5.20	5.20	15.0	107.1	69%	0.76	0.76	5%	5%	0.57								
FF15	2.26	9.60	9.60	15.0	107.1	82%	0.79	0.79	15%	90%	0.82								
H-H15	19.26	94.94	4.36	16.5	102.8	53%	0.71	0.78	2%	5%	0.57	2%	90%	0.82					
H15-H16	18.49	90.58	16.1		103.9	52%	0.71	0.78											
H16-H17	18.51	90.58	16.0		104.1	52%	0.71	0.78											
H17-H18	18.53	90.58	1.25	16.0	104.2	52%	0.71	0.78											
H18-H19	18.35	89.33	0.30	15.8	104.8	52%	0.71	0.78											
H19-H20	18.35	89.03	3.68	15.6	105.2	52%	0.71	0.78											
H20-H21	17.63	85.35	15.4		105.9	51%	0.70	0.78											
H21-H22	17.68	85.35	6.60	15.3	106.2	51%	0.70	0.78											
H22-H23	16.20	78.75	5.90	15.2	106.5	48%	0.70	0.78											
H23-H24	14.92	72.85	22.76	15.0	107.1	46%	0.69	0.66											
H24-H24A	10.47	50.09	2.76	15.0	107.1	51%	0.70	0.66											
H24A-H25	9.83	47.33	15.0		107.1	49%	0.70	0.66											
H25-H26	9.83	47.33	23.93	15.0	107.1	49%	0.70	0.66											
H26-H27	4.86	23.40	15.0		107.1	49%	0.70	0.66											
H27-H27A	4.86	23.40	11.28	15.0	107.1	49%	0.70	0.66											
H27A-H27	2.64	12.12	15.0		107.1	61%	0.73	0.66											
H27B-H27	2.64	12.12	0.80	15.0	107.1	61%	0.73	0.66											
H27C-H27	2.49	11.32	15.0		107.1	63%	0.74	0.66											
H27D-H27	2.49	11.32	15.0		107.1	63%	0.74	0.66											
H27E-H33	2.49	11.32	8.46	15.0	107.1	63%	0.74	0.66											

SECTION	RUNOFF	CATCHMENT AREA	ToC	INTENSITY	Equip	Corridors	LAND ZONING				See								
							Open Space	Commercial	Industrial	Special Uses		Residential							
UPS	Peak Flow [5 yr AR (cumecs)]	Cum. Increm. (ha)	Area [ha]	Kd5 = 612 m/s	612 mm/hr	612 mm/hr	% Area	% Corridors	% Area	% Corridors	% Area	% Corridors	% Area	% Corridors	% Area	% Corridors	% Area	% Corridors	
H35-H34	0.67	2.86	15.0	107.1	78%	0.78	0.66												
H34-H35	0.67	2.86	0.29	15.0	107.1	78%	0.78	0.70				100%	50%	0.70					
H35-H36	0.61	2.57	0.22	15.0	107.1	81%	0.79	0.83				100%	95%	0.83					
H36-H37	0.55	2.35	1.03	15.0	107.1	80%	0.79	0.79				100%	80%	0.79					
H37-H38	0.31	1.32	0.50	15.0	107.1	80%	0.79	0.79											
H38-H39	0.19	0.82		15.0	107.1	80%	0.79	0.79											
H39-H40	0.19	0.82	0.82	15.0	107.1	80%	0.79	0.79											
H24A-H24	0.63	2.76	2.76	15.0	107.1	75%	0.77	0.77				100%	75%	0.77					
H26-H45	3.57	16.46	8.99	15.0	107.1	60%	0.73	0.73											
H45-H46	1.62	7.47	2.83	15.0	107.1	60%	0.73	0.73				100%	60%	0.73					
H46-H47	1.01	4.64	3.15	15.0	107.1	60%	0.73	0.73											
H47-H48	0.32	1.49	1.49	15.0	107.1	60%	0.73	0.73											
H26-H49	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H49-H50	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H50-H51	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H51-H52	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H52-H53	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H53-H54	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H54-H55	1.40	7.47		15.0	107.1	26%	0.63	0.00											
H55-H55A	1.40	7.47	4.84	15.0	107.1	26%	0.63	0.70				100%	50%	0.70					
H55A-H48	0.60	2.63	0.53	15.0	107.1	74%	0.77	0.70											
H48-H48A	0.49	2.10	2.10	15.0	107.1	80%	0.79	0.79				100%	80%	0.79					
H55A-H55	0.80	4.84	4.84	15.0	107.1	0%	0.56	0.56				100%	0%	0.56					
H27A-H28	2.21	11.28	6.28	15.0	107.1	36%	0.66	0.66											
H28-H29	0.99	5.00	4.06	15.0	107.1	37%	0.66	0.66				100%	35%	0.66					
H29-H30	0.18	0.94	0.94	15.0	107.1	35%	0.66	0.66											
H29-H29A	0.07	0.30	0.30	15.0	107.1	70%	0.76	0.76				100%	70%	0.76					
H33-H42	1.82	8.46	1.00	15.0	107.1	58%	0.72	0.63											
H42-H42A	1.63	7.46	3.00	15.0	107.1	62%	0.74	0.63				100%	25%	0.63					
H42A-H43	1.07	4.46		15.0	107.1	87%	0.81	0.63											
H43-H44	1.07	4.46	4.46	15.0	107.1	87%	0.81	0.81				25%	90%	0.82					
												60%	85%	0.80	15%	50	90%	0.82	







**Stormwater Capacity Assessment**

**TABLE 2-5. RAINFALL INTENSITY and STORM EVENT CAPACITY LOOKUP  
JOHNSTONS CREEK SWC 55**

**(a) RAINFALL INTENSITY and VARIOUS FACTORS**

ARI years (y)	IFD Rainfall Intensity		Rainfall Intensity		Factors/Coef. with % <b>63%</b> and Duration = Avg of <b>23</b>			
	AR&R 1987 (Rainmar)		Equation Constants		Fdy	CoRdy	Ndy	Kdy . Fdy
	Uy4FId15r	Uy4FId60r	Kdy	mdy	Fdy	CoRdy	Ndy	Kdy . Fdy
0.5	47.08	22.83	296	0.614	0.435	0.581	0.342	219
1	64.49	31.27	406	0.614	0.596	0.629	0.507	325
2	83.09	40.71	509	0.605	0.770	0.676	0.705	438
3			555	0.595	0.868	0.704	0.827	497
4			587	0.588	0.941	0.723	0.922	540
5	107.10	53.95	612	0.582	1.000	0.739	1.000	575
6			626	0.579	1.035	0.751	1.052	599
7			638	0.576	1.064	0.762	1.097	619
8			649	0.573	1.090	0.771	1.138	636
9			658	0.571	1.114	0.779	1.174	652
10	121.03	61.85	667	0.570	1.135	0.786	1.207	667
12			687	0.567	1.180	0.798	1.276	698
15			712	0.564	1.237	0.814	1.362	737
20	139.39	72.09	745	0.559	1.311	0.833	1.479	790
25			769	0.557	1.366	0.849	1.570	830
50	163.41	85.66	844	0.548	1.543	0.896	1.872	962
100	181.68	96.09	917	0.540	1.720	0.943	2.196	1100

**(b) STORM EVENT CAPACITY (SEC) LOOKUP TABLE**

ARI years (y)	Lookup Factor		Storm Event Capacity Rating is (Alternative Display Options)					
	For Cap/q	With the						
	Ndy based average T	each section	Stand. Rat	At least	Colour	GIS Database	Group Rating	mdy
<0.5			Below.5	Below.5	Purple	0.25	< 0.5	0.614
0.5	0.342	219	0.5-1	0.5	Purple	0.50	< 1	0.614
1	0.507	325	1-2	1	Purple	1.00	1-2	0.614
2	0.705	438	2-3	2	Red	2.00	2-5	0.605
3	0.827	497	3-4	3	Red	3.00	2-5	0.595
4	0.922	540	4-5	4	Orange	4.00	5	0.588
5	1.000	575	5-6	5	Orange	5.00	5	0.582
6	1.052	599	6-7	6	Yellow	6.00	5-10	0.579
7	1.097	619	7-8	7	Yellow	7.00	5-10	0.576
8	1.138	636	8-9	8	Yellow	8.00	5-10	0.573
9	1.174	652	9-10	9	Light gr	9.00	10	0.571
10	1.207	667	10-12	10	Light gr	10.00	10	0.570
12	1.276	698	12-15	12	Light bl	12.00	10-20	0.567
15	1.362	737	15-20	15	Light bl	15.00	10-20	0.564
20	1.479	790	20-25	20	Grey	20.00	20-50	0.559
25	1.570	830	25-50	25	Grey	25.00	20-50	0.557
50	1.872	962	50-100	50	Grey	50.00	50-100	0.548
100	2.196	1,100	100 &>	100	Grey	100.00	> 100	0.540